

# Mooney Mooney & Peat Island Planning Proposal

Traffic & Transport Review

9 August 2021

Department of Planning, Industry and Environment (DPIE) NSW

383 Kent Street Sydney NSW 2000 PO Box Q1678, QVB Sydney, NSW 1230 Australia

T +61 (0)2 9098 6800 F +61 (0)2 9098 6810 mottmac.com

4 Parramatta Square, 12 Darcy Street, Parramatta NSW 2150

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#### Information class: Standard

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# **Executive summary**

Mott MacDonald has been engaged by Property & Development NSW (P&D NSW) to provide transport planning and engineering services to support the Planning Proposal for the rezoning of P&D NSW land at Mooney Mooney and Peat Island (The Site). This report aims to address all key traffic and transport related matters associated with the proposed Concept Plan and specifically address comments received from the Council, Transport for NSW (TfNSW) and various government agencies following the submission of the Traffic and Transport Review of the development in September 2016 and December 2018.

The proposed development will comprise a mixture of land uses including residential and hospitality. It is expected to generate and attract less than 300 new trips in each peak hour. Those trips are associated with the proposed development.

Analysis of the Australian Bureau of Statistics (ABS) Journey to Work 2016 data indicates that travel by private car is the dominant travel mode for journeys in Mooney Mooney. Trains also attract a significant number of commuters with a 12 percent travel mode share, while buses don't seem to be as attractive with less than one percent of trips made by buses.

Initial investigations identified gaps in the cycling and walking network within The Site, with very limited walking and cycling facilities. Improvements to active transport links and facilities could improve the utilisation of both walking and cycling as travel modes within the development.

A review of available capacity of rail services was undertaken to understand the potential impact on current peak hour commuter services by the proposal. The review indicated that existing rail services operating via Hawkesbury River station operate with some spare capacity and should easily accommodate any additional demand generated by the proposal.

Bus services are infrequent in nature due to the existing low demand, and The Site will therefore have no negative impact on bus services, however, increase in bus routes and frequency will improve the public transport amenity of The Site when the development is could be supported by upgrades to the bus stop facilities within The Site.

Road network performance has been assessed for the current situation and the future 2030 scenario with and without development, to understand the impact of the vehicular trips generated by the development. Intersection analysis was undertaken using SIDRA for the major local intersections within the study area. Additionally, motorway segments including on and off ramps of the Pacific Highway (M1) were assessed using static calculations by applying the Highway Capacity Manual (HCM 2010) criteria. Analysis showed that all intersections and motorway segments are currently operating at very acceptable Level of Service (LoS) and will continue to do so under the future 2030 scenario after the completion of the development.

Various opportunities related to both public transport and active transport have been identified as part of the developed transport strategies and those are detailed in this report.

# **1** Introduction

#### 1.1 **Project Appreciation**

This Planning Proposal has been prepared on behalf of Property & Development NSW that seeks amendments to the Gosford Local Environmental Plan 2014 (GLEP 2014) for surplus Government owned land at Peat Island and Mooney Mooney (the Site).

The aim of the Planning Proposal is to facilitate the future redevelopment of the site, for a mix of residential, community, tourism and employment generating land uses.

This Planning Proposal was first submitted to Central Coast Council in November 2016. Gateway Determination was issued by the Department of Planning, Industry and Environment (DPIE) on 10 August 2017 (PP\_2017\_CCPAS\_006\_00 (17/06254). The Gateway Determination stated that while the supporting studies were sufficient, a number of conditions are required to be addressed prior to progressing the Planning Proposal further. Since August 2017, Property & Development NSW has undertaken a significant amount of consultation with public authorities and Central Coast Council (Council), including the submission of a revised Planning Proposal to Council in December 2018 for review and comments.

Post the 2018 submission, Property & Development NSW has engaged technical consultants to undertake further environmental investigations to respond to Council's and public authorities feedback.

The indicative Concept Plan has been revised in accordance with the additional technical investigations post 2018 submission. The revised indicative Concept Plan comprehensively evaluated the additional environmental and physical constraints, and responded to site's context, future amenity and connectivity.

The revised indicative Concept Plan is attached at Appendix A.

Lot 9 DP 863305 is excluded from the Planning Proposal, given it is under the care, control and management of Central Coast Council and will be retained as RE1 Public Recreation Zone. The indicative Concept Plan identifies a proposed Rural Fire Services (RFS) at this location. This RFS facility does not form part of this Planning Proposal, and is subject to further stakeholder consultation and a separate planning proposal.

The indicative Concept Plan also identifies a proposed location for a Marine Rescue NSW facility. This facility is subject to further stakeholder consultation and a separate proposal.

Private Recreation land is shown on the Indicative Concept Plan located on the foreshore of the Hawkesbury River adjacent to Peat Island. For the purposes of this assessment, it is assumed that this land use will operate as a dry stack boat storage. However, it does not form part of this planning proposal and would be subject to a separate future planning proposal if it is to proceed. This would include a detailed environmental assessment of the impacts.

This part of the site is currently zoned partly RE1 Public Recreation and partly SP2 Infrastructure (for the purpose of hospital) under GLEP 2014, and is proposed to be rezoned to RE2 Private Recreational Zone. A car park is proposed to be an Additional Permitted Use under Schedule 1 of GLEP 2014 on a portion of the site as part of the Planning Proposal.

This Transport and Traffic Assessment Report has been prepared based on the revised indicative Concept Plan and the draft LEP zoning maps.

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#### PROPOSED PLANNING CONTROL AMENDMENTS

The Planning Proposal is seeking to amend the following provisions of the GLEP 2014:

- Amend Clause 2.1 Land Use Zones of the GLEP 2014 to include SP3 Tourist zone listed under Special Purpose Zones. The proposed SP3 Tourist Zone objectives and proposed permissible uses are consistent with the draft SP3 Tourist zone within the draft Consolidated Central Coast Consolidated Local Environmental Plan (CCLEP). Therefore, this Planning Proposal will be consistent with draft CCLEP, subject to gazettal.
- Amend the GLEP 2014 Land Zoning Map applicable to the site, and rezone SP2 Infrastructure and RE1 Public Recreation zones to E2 Environmental Conservation, R1 General Residential, R2 Low Density Residential, RE1 Public Recreation, RE2 Private Recreation, and SP3 Tourist zones.
- Amend the GLEP 2014 Height of Buildings Map to reflect the maximum height of the buildings proposed (8.5m, 12m and 15m) across selected areas of the site as indicated on the proposed Height of Buildings Map.
- Amend the GLEP 2014 Lot Size Map to allow minimum lots size of 150sqm, 220sqm, 300sqm and 450sqm across selected areas of the site as indicated on the proposed Minimum Lot Size Map.
- Amend the GLEP 2014 Additional Permitted Uses Map and amend the GLEP 2014 Schedule 1 Additional permitted uses to include the use of certain land at Mooney Mooney, including:
  - RE2 Private Recreation zoned land, being portion of Lot 11, DP 1157280 and Lot 12, DP 1158746 as identified on the Additional Permitted Uses Map.
    - To include 'car parks' as additional permitted use on this part of the site.
    - R1 General Residential zoned land, being the southern portion of Lot 14, DP1158746 as identified on the Additional Permitted Uses Map.
      - Development for the purposes of emergency services facility is permitted with development consent. The proposed emergency services facility is permissible with consent within the proposed R1 General Residential zone under the draft CCLEP. Therefore, this Planning Proposal will be consistent with draft CCLEP, subject to gazettal).
  - RE1 Public Recreational zoned land, being the southern portion of lot 4 DP239249 as identified on the Additional Permitted Uses Map.
    - Development for the purposes of emergency services facility is permitted with development consent. The proposed emergency services facility is permissible with consent within the proposed RE1 zone under the draft CCLEP. Therefore, this Planning Proposal will be consistent with draft CCLEP, subject to gazettal.
  - R1 General Residential zoned land, being the south eastern portion of lot 12, DP1158746 located along Peats Ferry Road, lot 12, DP863305 and the southernmost portion of lot 14DP1158746, as identified on the Additional Permitted Uses Map:
    - Development for the purpose of 'food and drink premises' and 'shops' are permitted with development consent.
    - The indicative Concept Plan comprises local shops/restaurants and cafes in the form of shop top housing within the Southern Foreshore precinct and the Chapel precinct, which has an area of approximately 200sqm. The proposed shops and food and drinks premises are of a scale that is better suited for this local area. Shops. Restaurants and cafes are prohibited under the R1 zone of the Gosford LEP and the draft CCLEP. Given the proposed to include food and drink premises and local shops to provide sufficient and much needed local retail services for exiting and incoming residents.
  - RE1 Public Recreation zoned land, being Lot 11 DP863305 as identified on the Additional Permitted Uses Map.
    - Development for the purpose of electricity generating works is permitted with development consent.

In addition, consistent with the recommendation of the CMP, this Planning Proposal includes the proposed LEP amendment to include Peat Island as an Item of Environmental Heritage (Item - General) under Part 1 - Heritage Items, Schedule 5 of the Gosford LEP.

## 1.2 **Project Transport Assessment**

Mott MacDonald has been engaged to provide traffic engineering services to support the Planning Proposal to rezone State Government owned land at Mooney Mooney and Peat Island (The Site).

A previous planning proposal was submitted to the Department of Planning and Environment (DPE; now Department of Planning Industry and Environment) for consideration in 2014. As part of the proposal a Traffic, Transport and Access Report was undertaken by GTA Consultants. Comments from the review by DPE indicated that further consideration of the suitability of The Site in terms of existing and future traffic and transport planning and supporting measures needed to be explored and addressed to inform the proposed rezoning of the area and the establishment of new planning controls.

The DPE comments and concerns were addressed in the Traffic and Transport Review that was undertaken by Mott MacDonald in 2016. Following the submission of this report in 2016, further comments were received from the Council, Transport for NSW (TfNSW), and various other government agencies which were addressed. A further updated Concept Plan was developed in July 2021 and is the basis of this current assessment.

### 1.3 Site Location

The Mooney Mooney and Peat Island site (refer to Figure 1.1) is located on the shores of the Hawkesbury River and adjacent to the M1 Motorway. It is well-connected to nearby regional, sub-regional and local centres by both road and rail.

In terms of its locality to surrounding centres it is situated approximately 50 km north of Sydney CBD, 24 km north of Hornsby rail station, and 2.5km north of Brooklyn. To its north are established regional townships situated at Gosford (approximately 27 km north) and Wyong (approximately 45 km north by road).

The Site has the potential to offer a good level of local and sub-regional access and the area itself provides the opportunity to capture recreational and lifestyle attractions and to support continued growth in NSW tourism.

Refer to Appendix A for further details of The Site, including the Concept Plan.

### 1.4 **Proposed Rezoning**

An updated Concept Plan has been developed for The Site. The Concept Plan includes a mix of community, residential and hospitality generating uses, as shown in the Concept Plan provided in Appendix A.

This report will identify opportunities and address potential issues associated with supporting access, movement and the overall integration of the proposed site with its surroundings as part of the proposed rezoning of The Site.

#### **1.5 Purpose of the Report**

The purpose of this report is to assess the potential traffic and transport impacts of the revised Concept Plan, and to address relevant traffic and transport issues identified in previous planning and traffic and transport review submissions. The analysis was carried out at an appropriate level that helps to inform rezoning of the land and the establishment of planning controls.



Figure 1.1: Mooney Mooney and Peat Island Location Plan

Source: Google Maps (2016)

### 1.6 Assumptions and Report Limitations

Our assessment is based on and is limited to the following assumptions and limitations:

- The assessment was based on traffic generation rates as stated in the RMS Guide to Traffic Generating Developments (2002) and the more recent RMS's Technical Direction 2013/04a: Guide to Traffic Generating Developments – Updated Traffic Surveys (RMS, 2013) and are generally conservative given the characteristics of planned uses.
- Traffic distribution was based on information extracted from ABS Journey to Work data for the Mooney Mooney area dated 2016.
- The network assessment was carried for Weekday AM and PM peak periods and the Weekend peak period. Traffic volumes for these periods around the Site were obtained from traffic surveys and the TfNSW permanent count stations on the M1 Pacific Motorway (2017).
- The road safety assessment was limited to the TfNSW crash statistics (January 2013- December 2017) provided by TfNSW.
- The concept plan land use was limited to the detail provided in the preferred concept plan for the rezoning of The Site.
- The assessment is based on an existing situation, 2030 without development scenario and full development preferred concept plan scenario, and is limited by the data obtained and identified in this report.
- The assessment of The Site and concept is based on the existing situation and the horizon year 2030.
- The assessment was carried out at a high level using SIDRA modelling software for the local intersection within the development for both weekdays and weekend in addition to operational assessment of the motorway segments including the on and off ramps of the M1 using the Highway Capacity Manual (2010) method.

#### 1.7 Report Structure

The remainder of this report is structured as follows:

- This Section introduces the project and the aims and limitations of the report.
- Section 2 covers the background of the project, previous studies and findings, relevant guidance obtained from consultation with other Government agencies and the project alignment with strategic planning objectives.
- Section 3 provides an understanding of the existing situation including area and network characteristics and the service and facility conditions.
- Section 4 Provides a broad overview of the preferred concept plan and proposed uses.
- Section 5 provides an assessment of the road network and local intersections under the current situation and the future 2030 scenario.
- Section 6 covers the pedestrian and cycling strategy.
- Section 7 outlines the transport management and access strategy.
- **Section 8** outlines traffic management strategy for the local area.
- Section 9 provides an overview of the wayfinding strategy.
- Section 10 summarises the key findings.

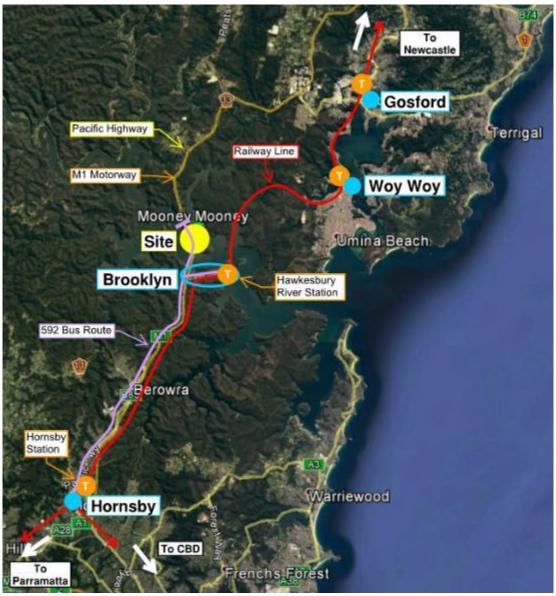
# **2** Background and Planning Context

This section provides an overview of the strategic context, project proposal and previous studies, and the project's alignment with Government planning and policy goals.

### 2.1 Strategic Context

The strategic context of The Site in relation to nearby centres and transport routes is presented in Figure 2.1.

#### Figure 2.1: Strategic Context



Source: Google Earth (2016)

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Figure 2.1 demonstrates that The Site is well connected to nearby centres via the M1 Motorway, Pacific Highway and T1 North Shore train line. The 592 bus route also connects The Site to the south including Brooklyn (with the Hawkesbury River train station) and Hornsby.

# 2.2 Alignment with NSW 2021, Future Transport Strategy 2056, and the Central Coast Regional Plan

The key guiding documents that are developed by Government for the rezoning of The Site are NSW 2021 – a Plan to Make NSW Number One (2011), Future Transport Strategy 2056 (2018) and the Central Coast Regional Plan 2036 (2016).

*NSW 2021* is identified to be NSW 10–year plan for guiding policy direction, budget decision making and delivering on community priorities. It sets long–term goals and measurable targets, and outlines immediate actions that will help state growth, and to improve opportunities and quality of life for people situated in both regional and metropolitan areas of NSW.

The *Future Transport Strategy 2056*, which builds upon the previous 2012 NSW Long Term Transport Master Plan and the commitments it has delivered, is an integrated transport strategy for NSW that has a key focus on movement and place, bringing together land use and transport planning which aims to improve the integration of all modes of transport. It proposes to develop target goals and action plans that set a clear direction for all transport modes and establish a path that offers an enhanced transport system that can meet current and future customer needs and support projected growth.

These goals and actions can be supported by the proposed rezoning of The Site, which is demonstrated in the subsequent sections of this report. The travel demand management plan identified in the previous submission together with the transport and traffic related strategies included in this report will help to formulate an appropriate package of measures that help to maintain network reliability, maximise the potential of existing infrastructure and services, and support and promote travel by public transport, walking and cycling, and improving safety.

The *Central Coast Regional Plan 2036* (DPIE, October 2016) provides a regional planning vision for the Central Coast. Due to the early planning status of this rezoning application, The Site itself is not specifically mentioned, however in general terms it is covered through strategic directions that aim to increase residential development through infill and the availability of existing infrastructure and services. This aims and aligns with the planning concept through encouraging increases in housing supply and choice in established areas, which will make best use of existing services and infrastructure such as public transport.

# 2.3 Alignment with Ministerial Direction

The applicable Ministerial Directions under section 9.1 (2) of the Environmental Planning and Assessment Act indicate the following:

### • Ministerial Direction and Best Practice 3.1 – Residential Zones

The Ministerial Direction indicates that development proposals should make efficient use of existing infrastructure through locating near to established infrastructure, such as city centres, neighbourhood centres, transport hubs, schools, employment precincts, recreational facilities and regional services.

This proposal is identified to align with these principles and provides good connectivity to established and planned employment, education, retail and recreation facilities. This is achieved through existing services and network infrastructure opportunities, which offers access travel mode choices for both local and regional travel. Its proximity to the above can help to maximise the potential of existing infrastructure and support access by public transport, walking and cycling. Refer to sections 2.1, 6, and 7 for further details.

#### • Ministerial Direction and Best Practice 3.4 – Integrating Land Use and Transport

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The Ministerial Direction indicates that the development proposals should have a positive contribution to managing travel demand.

The Site provides an integrated land use and transport solution that aligns with the direction, and the planning principles set out in NSW 2021, Draft Central Coast Regional Plan and the NSW Long Term Transport Master Plan. Its position in the Hawkesbury River catchment can help to support travel by walking, cycling and an existing public transport system. The inclusion of residential density at this site provides an opportunity to support growth while reducing car dependency in the Hawkesbury River catchment. The site is situated within walking distance of a new local centre and established bus service route stops, and within cycling distance of an established township with a railway station.

# 2.4 Department of Planning and Environment Queries and Consultations with RMS and TfNSW

Traffic and transport issues raised by the DPE (now DPIE) are presented in Appendix B together with how these elements were addressed as part of Traffic and Transport Review report that was issued in September 2016.

Additionally, extensive consultation was undertaken with different departments of RMS (now TfNSW) and TfNSW prior to issuing the report in September 2016 and the key issues raised as part of this process are presented in Appendix B.

Following the submission of the Traffic and Transport Review report on the 12<sup>th</sup> of September 2016, further issues and queries were raised by the Council, TfNSW, RMS and other government agencies.

Lists of issues raised and responses to each has been included in Appendix C of this report.

Further consultation with RMS/TfNSW related to a proposed service station provided on a parcel of land adjacent to the M1 occurred between 2018 and 2019 (based on a previous layout). Following a number of updates to both the network modelling and reporting, as well as detailed consultation with TfNSW, it was decided that a change to the masterplan layout would be required to resolve the outstanding issues raised, which included the removal of development of the service station site from the Planning Proposal.

Relevant correspondence through this period has been provided in Appendix K of this report.

# **3 Existing Conditions**

#### 3.1 The Site

The Site is separated by the M1 Pacific Motorway (M1) corridor, which is a high-speed interstate corridor with Average Annual Daily Traffic (AADT) volumes of approximately 42,000 vehicles in each direction in 2017 as contained in the TfNSW traffic volume viewer. Access to and from the M1 is via on/off ramps that offer access in both northbound and southbound directions. An established rest area that is positioned on the eastern side (southbound direction) of the corridor is currently used by passing traffic and regularly used by heavy vehicle drivers. This corridor restricts access between the western and eastern sections of The Site, which are limited to a two-lane road corridor running under the M1 and a pedestrian underpass further north. Refer to Figure 3.1 for further details.

The Mooney Mooney and Peat Island local catchment is characterised with the following uses:

- Small pockets of low-density residential housing;
- Disused or infrequently used facilities (Peat Island, TfNSW depot, RFS depot, and a Chapel);
- A public boat ramp at the southern end of the peninsula;
- A limited number of commercial activities on the eastern foreshore;
- The Mooney Mooney Club (off Kowan Street);
- Deerubban Reserve to the southern edge of the area; and
- Brisbane Water Nation Park to the north.

Large parts of The Site are currently zoned SP2 (Special infrastructure) for either road, hospital or educational purposes. In most of cases these uses are now surplus to requirements and Government (the land-owner) is currently seeking to rezone the land for more appropriate uses that could support regional strategies and growth of the economy.



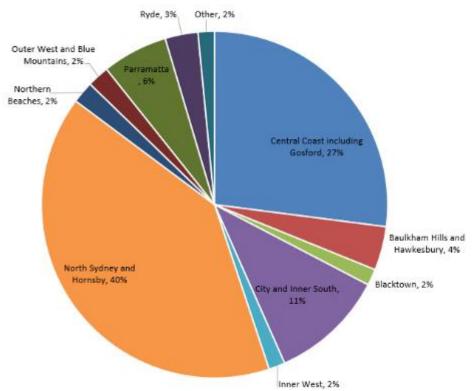
Figure 3.1: Snapshot of the Existing Situation – The Site

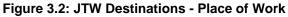
Source: Google Maps (2016) combined with Mott MacDonald edits (2018)

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## 3.2 Current Travel Characteristics

An analysis has been undertaken of 2016 Australian Bureau of Statistics (ABS) Journey to Work (JTW) trends for people currently residing in Mooney Mooney. Figure 3.2 shows the normal place of work for people living in Mooney Mooney.



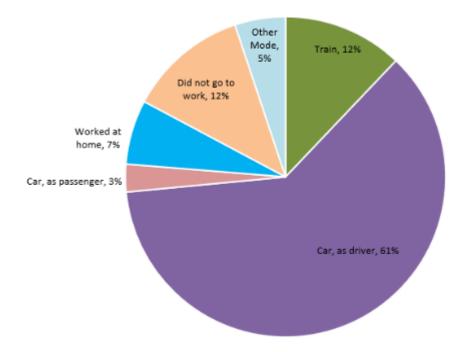


The data indicates that people currently residing in the study area work in a variety of commercial centres and employment areas, with Gosford and Hornsby being key destinations followed by the Sydney CBD, inner south, and Paramatta.

Figure 3.3 shows the normal JTW travel mode for people living in Mooney Mooney. According to ABS data (2016), 215 persons travel to work.

Source: ABS Census Data Journey to Work (2016)

# Figure 3.3: JTW Travel Mode



Source: ABS Census Data Journey to Work (2016)

The data indicates that travel by private car is the dominant travel mode for journeys in Mooney Mooney. 64 percent of residents travelled by car as either a driver (61 percent) or a passenger (three percent) and this reflects the quality of access. Trains also attracts a significant number of commuters with a 12 percent travel mode share. The data also highlights that some residents did not go to work (12 percent) and indicates that this area is attractive to retirees.

Hawkesbury River station offers services to the major centres of Gosford and Hornsby (the key journey to work destinations). It is noted that the latest 2016 JTW data shows that none of the people surveyed travelled to work by bus and that road (driving) and rail are the main modes of access for commuting.

### 3.3 Road Network

The catchment is supported by the following road characteristics and the road network details highlighted in Figure 3.1:

#### 3.3.1 M1 Pacific Motorway

The M1 (previously the F3 Freeway) is a major state road that connects key urban centres and conurbations, including Sydney, the Central Coast and Newcastle. This important arterial road link contains three lanes in each direction and includes entry and exit ramps at Mooney Mooney in both directions. The posted speed limit along this section of the motorway is 110 km/h.

#### 3.3.2 Pacific Highway

The Pacific Highway is a historical strategic road link that was replaced in this section by the M1 and functions as a regional road providing access to surrounding suburbs and recreation areas. This section of the Pacific Highway provides access to Brooklyn to the south (including Hawkesbury River train station) and Cheero Point to the north and contains one lane in each direction and generally does not allow for kerbside parking. The posted speed limit through Mooney Mooney is 60 km/h, which increases to 80km/h over the Hawkesbury River.

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#### 3.3.3 Pacific Highway Link Road

The Pacific Highway Link Road functions as a collector road connecting the Pacific Highway and M1 Motorway northbound off and on ramps. It allows access to and from Mooney Mooney for northbound traffic on the motorway and also feeds into local and private roads on the western section of The Site. The Pacific Highway Link Road contains one lane in each direction and has a posted speed limit of 60 km/h.

#### 3.3.4 Peats Ferry Road

Peats Ferry Road is a local access road that services parklands, an existing boat ramp and a car park. It contains one lane in each direction and does not have a posted speed limit.

### 3.4 Bus services and facilities

The Site catchment is served by one bus service route (592) that travels along the Pacific Highway and is operated by Transdev. The Route 592 provides an important public transport link to this part of the Hawkesbury River catchment and serves:

- The townships of Cheero Point (residential) and Mooney Mooney (residential/ recreational/ tourist village) to the north of the Hawkesbury River; and
- Brooklyn (Hawkesbury River rail station and recreational/tourist village), Cowan (rail station), Berowra (rail station and village centre), and Hornsby to the south.

The access offered by this service was identified to be very important for this catchment through its ability to offer direct access from The Site to other public transport and regional rail connections. This includes the nearby rail stations at Brooklyn that offers fast and reliable connections to regional centres at Gosford and Hornsby, and the metropolitan centres of Sydney and Newcastle. Each of these centres offer employment, health, education, retail and recreation opportunities for existing and proposed residents of Mooney Mooney area.

Refer to Appendix D for an understanding of the bus service route and its connectivity to rail stations and other surrounding local centres.

The review of 592 service operations indicated that it offers an infrequent weekday and Saturday service that is focused around peak commuter periods. This service in general offers limited access opportunities and based on the current timetable does not appear to service weekday and Saturday off peaks, Sundays or public holiday periods. Stopping patterns of the service also appears to be inconsistent with some stops, such as the Peat Island Road bus stop only receiving one service a day. This is based on a new timetable introduced by TfNSW in November 2018 and has been in effect since 2<sup>nd</sup> December 2018.

Route Number	Route Description	Peak Period (7-9AM, 4-6PM)	Off-Peak Period
592	To Brooklyn Hawkesbury River Station	Ranges between 25 to 50 minutes	
592	From Brooklyn Hawkesbury River Station	Ranges between 25 to 40 minutes	

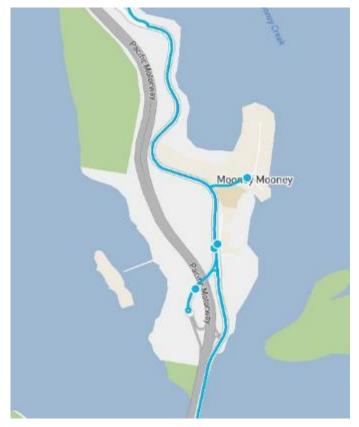
#### Table 3.1: Bus frequency

Source: transportnsw.info/routes/bus

There are four bus stops within The Site, two are located on the Pacific Highway near Kowan Road, one is located on the Pacific Highway Link Road near Peat Island and one is on Point Road near Mara Crescent close to Mooney Mooney Public School. The location of the bus stops is as shown in Figure 3.4 below.

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Figure 3.4: Bus Stops Locations



Source: Transportnsw.info

Bus stop facilities were identified to be sub-standard with stops not supported by an established footpath network, shelters, bus timetable and passenger information (except for the bus stop on Point Road) or Disability Discrimination Act (DDA) facilities as shown in below Figure 3.5 and Figure 3.6.

#### Figure 3.5: Bus Stops on the Old Pacific Highway and the Pacific Highway Link Road



Source: Google earth (2018)

#### Figure 3.6: Bus Stop on Point Road



Source: Google earth (2018)

#### 3.5 Rail services and facilities

The nearest rail station to The Site is the Hawkesbury River station in Brooklyn, which offers Sydney-Newcastle train services. This is situated approximately 5.5 kilometres to the south of The Site by road and offers relatively frequent services to employment and education facilities situated at Sydney, Hornsby, Chatswood, Macquarie Park and Newcastle. The current frequency of rail services is summarised in Table 3.2.

#### **Table 3.2: Train frequencies**

Route Description	Peak Period (7-9AM, 4-6PM)	Off-Peak Period
To Newcastle	One every 60 minutes (AM) One every 30 minutes (PM)	One every hour
To Sydney	One every 30 minutes (AM) One every 60 minutes (PM)	One every hour

Source: transportnsw.info/routes/train

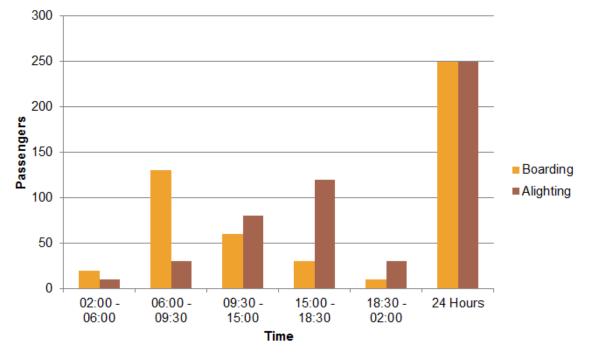
Access to the Hawkesbury River station is possible by private vehicle, bicycle and bus services. However, it is noted that both bus service frequency and parking provision at the station and township are limited as shown below in Figure 3.7. It is also noted that the bus timetable doesn't completely tie-in with the train one, however, there is an average 15-minute difference between buses arriving at Hawkesbury River Station and the train services leaving the station in the PM. In the AM peak, the difference could be anywhere from seven minutes to 50 minutes depending on the bus arrival time at the station. Cycling to the station is influenced by the quality of supporting infrastructure and is also noted to be limited.



Figure 3.7: Limited Parking at Hawkesbury River Station

Source: Google earth (2018)

In order to obtain an understanding of the function of the station and capacity of existing services a review of the daily patronage profile was undertaken. Figure 3.8 provides a breakdown of typical weekday passenger movements at Hawkesbury River station.



#### Figure 3.8: Weekday Passenger Flow Profile at Hawkesbury River Station (2012)

Source: RailCorp Barrier Counts of Rail Passengers (2012)

The passenger profile indicates that Hawkesbury River station acts as a minor commuter station for passengers situated along the Newcastle and Central Coast line and offers spare capacity to support future growth. Refer to Section 5.2 for an understanding of available spare service capacity.

A review of the station's compliance with current DDA standards was also considered as part of the assessment, which highlighted (based on information contained on the TfNSW website) that the station is not DDA compliant as shown in Figure 3.9 below. The station is only accessible via stairs, with no wheelchair access provided. The nearest station that offers access for people with DDA needs is Cowan and can be accessed via the 592-bus service route.

Figure 3.9: Access to the Hawkesbury River Train Station (Not DDA compliant)



Source: Google earth (2018)

## 3.6 Ferry Services

The nearest wharf for Ferry services from The Site is the Brooklyn Wharf which is located approximately 200 m east of the Hawkesbury River Train Station.

Brooklyn Ferry Service is a small private ferry company operating under contract for TfNSW. The company operates two restored historic vessels between Brooklyn, Dangar Island and Little Wobby. The ferry service provides essential public transport for commuters and tourists in these areas.

The ferry runs every day to a timetable and, where possible, links with trains arriving and departing from Hawkesbury River railway station. Typically, the ferry service runs at a frequency of 30 to 60 minutes on all weekdays with the services terminating at 20:00 and 19:00 on weekends. The service between Brooklyn and Dangar Island is more frequent than the Little Wobby service. Figure 3.10 below shows a view of Brooklyn Wharf.

#### Figure 3.10: View of Brooklyn Wharf



Source: Google earth (2018)

# 3.7 Walking and Cycling

There are currently a limited number of footpaths and bicycle facilities within The Site area and as a result, current opportunities for cycling beyond commuter or recreational uses are limited. A review of Central Coast Council's "Gosford Bike Strategy 2014" and subsequent discussions with Council confirmed that there are no commitments to improve cycle routes or other facilities in the Mooney Mooney area. However, it is noted that the Gosford Bike Strategy 2014 states a goal of increasing the cycling mode share, which is consistent with the NSW State Plan and a commitment to improving road safety, facilities and cycling tourism.

The existing active transport network is presented in Figure 3.11.



Figure 3.11: Existing Active Transport Network

Source: TfNSW (https://roads-waterways.transport.nsw.gov.au/maps/cycleway\_finder)

# 3.8 Existing Traffic Data

Traffic counts were undertaken in September 2018 for both a weekday AM and PM peak periods, which consisted of a Tuesday counts collected between 05:00 to 08:00 and 15:00 to 18:00 in addition to a weekend peak period which consisted of a Sunday counts collected from 08:00 to 18:00. The intersections appraised are shown in Figure 3.12 and described below:

- 1. Old Pacific Highway/ B83 Pacific Highway operates as a give-way controlled T-intersection;
- 2. M1 Northbound ramps/ Peats Ferry Road/ B2 Site Road/ Pacific Highway Link Road operates as a four-way roundabout; and
- 3. M1 Southbound ramps/ B83 Pacific Highway operates as an all way stopped controlled intersection under normal operations with the ability to operate under traffic signal control (temporary operations).

# Figure 3.12: Modelled Intersections



Source: Google Earth (2018)

These intersections represent the key access points to the Mooney Mooney area from the M1, Pacific Highway and the local network.

The analysis of data indicated that 06:30-07:30 and 15:00-16:00 represent the weekday AM and PM peak hours for the Mooney Mooney area. Surveys undertaken on Sunday to determine the impact from weekend traffic showed that the peak hour occurs between 11:30 to 12:30.

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A review of traffic data on the M1 was also undertaken, which was extracted from a permanent count stations at Cowan to the south of The Site and Cheero Point to the north. Location of the traffic count stations are shown in Figure 3.13.



#### Figure 3.13: Location of Permanent Count Stations

Source: Traffic Volume Reviewer (Source: TfNSW)

The review of this data clarified that the peak hours on this section of the network are 06:00 to 07:00 and 16:00 to 17:00 on a weekday and 11:00 to 12:00 on the weekend.

Existing traffic data was used to develop a SIDRA model to assess existing intersection performance in addition to assessing the performance of the motorway segments of the M1 including the on and off ramps from and to Mooney Mooney area as shown in Figure 3.14 below. The motorway segment performance was undertaken using the Highway Capacity Manual HCM (2010).

Refer to Section 5.3 for the results of the assessment and comparison with the results under a scenario including the rezoning proposal and full development of The Site.

Figure 3.14: Analysed Motorway Segments



### 3.9 Crash Data Analysis

Crash data was obtained for a five-year period from January 2013 to December 2017 to understand the crash history in the area and a summary is presented in Appendix E. The data set used in the analysis is as shown below:

- Crash dataset 8317 Pacific Highway, between Kangaroo Point and Point Road, Mooney Mooney Crash Data - 1 Jan 2013 to 31 Dec 2017;
- Crash dataset 8317 Pacific Motorway (including off/on ramps), between Kangaroo Point and the southbound safety ramp, Mooney Mooney Crash Data 1 Jan 2013 to 31 Dec 2017; and
- Crash dataset 8317 Northbound Pacific Motorway off/on ramp, up to intersection with Pacific Highway, Mooney Mooney Crash Data 1 Jan 2013 to 31 Dec 2017.

The key findings of the analysis are listed below:

 68 percent of crash-movements are rear-end movements. This is likely to occur when the Pacific Motorway is congested;

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- 70 percent of rear-end crashes occur close to the southern Pacific Motorway ramps;
- Cars are involved in 89 percent of crashes, while light trucks are involved in 42 percent of crashes;
- Weather and visibility conditions do not appear to affect crash rates. Most crashes occur with fine weather and dry conditions;
- Crashes are not significantly increased during holiday periods or weekends;
- The rate of crashes increases during peak hours, likely as a result of higher traffic volumes during these periods; and
- No fatal injuries occurred from 1 Jan 2013 to 31 Dec 2017. Moderate and minor injuries account for almost 85 percent of all injuries. The average number of casualties is 0.78 for each crash.

Figure 3.15 and 3.16 highlight the crash movement type and level of injury for each recorded crash in the five-year period.

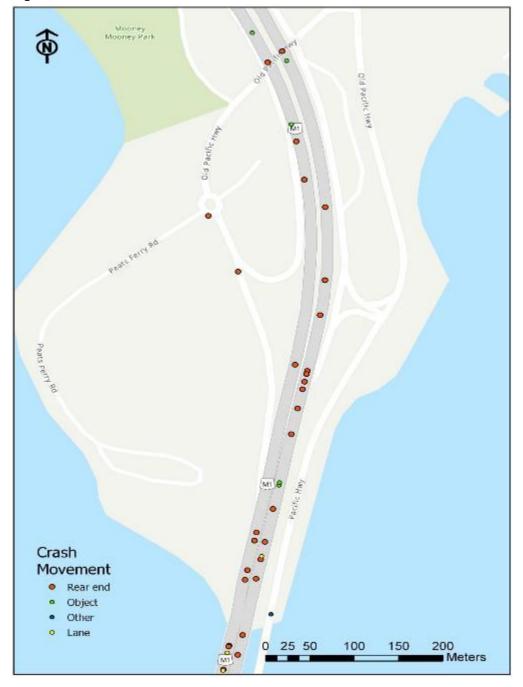


Figure 3.15: Crash Movement based on Crash Location

Source: Roads and Maritime Services, Crash dataset 8317 - Mooney Mooney Crash Data - 1 Jan 2013 to 31 Dec 2017

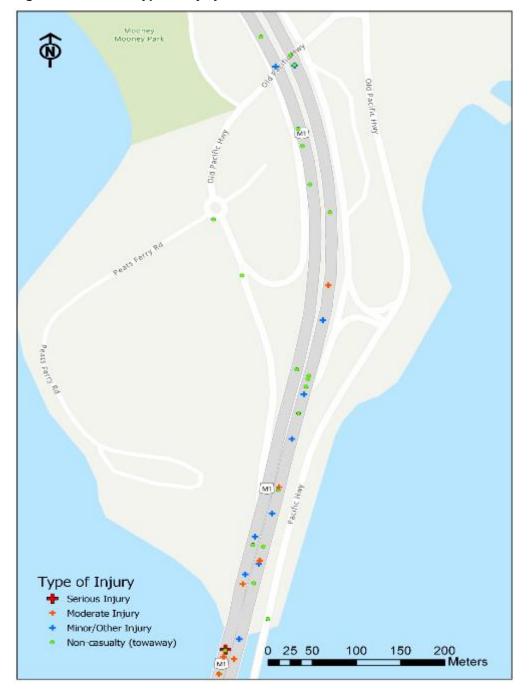


Figure 3.16: Crash Type of Injury based on Crash Location

Source: Roads and Maritime Services, Crash dataset 8317 - Mooney Mooney Crash Data - 1 Jan 2013 to 31 Dec 2017

It is noted that the crash trend in the area is unrelated to the existing land use and therefore should be addressed as part of an operational solution of the M1 corridor.

Further investigation into the cause of crashes and planned improvements by Government in traffic flow, monitoring and warning management systems are expected to help address this current road safety issue.

The proposal could be configured to help support a design solution that would be addressed during DA phase for this existing issue through the visual appearance of development, separation, and improvements in wayfinding systems and access routes between the western and eastern sides of the corridor. It is also noted that the proposal has the potential to exacerbate the issue if not addressed as part of the above, which is expected to be a focus of Governement under the NSW 2021 targets and goals.

Refer to Appendix E for further details of the crash analysis.

# 4 Concept Plan

This section details the key components of the Concept Plan. Refer to Appendix A for the proposed Concept Plan.

The proposed development will comprise of a mixture of land uses including residential and hospitality. The key components are described below.

Proposed Component	Description
Adaptive reuse of the	Intended for hotel/accommodation purposes
existing Peat Island institutional buildings	Constrained by Peat Island causeway and is intended that vehicular access is restricted to service vehicles
	The hotel parking requirement (location 6) is provided within the at- grade car park adjacent to the buildings at location 4
Private recreation	Located to the south of the Peat Island causeway
	Includes parking provision.
	Assumed to be indicative dry-stack boat storage (subject to investigation and separate proposal).
New medium and low- density residential areas	Will accommodate residential developments for pre-retirement/ semi-retired/ fully retired housing markets with a focus on attracting people seeking recreational riverside lifestyle
	Offers an opportunity for contained travel during peak periods and encourages public transport use due to the proximity and access to nearby centres and rail services
Recreational paths and active transport connectivity	Improvement of the river foreshore to a shared path from the northern residential area to the existing boat ramp at the southern peninsula
	Will also connect to the new residential areas, dry-stack boat storage (subject to separate proposal) and community facilities
	Includes a potential east-west connection via the M1 underpass or Pacific Highway Link Road, and a potential extension to Brooklyn and Hawkesbury River train station
Public parklands and public car parking	Provided throughout the western section of The Site and supports proposed land uses

Table 4.1 provides details of the master plan land use.

#### Table 4.1: Concept Plan Land Use Summary

•						
	Zoning Land Area		# Dwellings		GFA	Other
ZONE	(sqm)	Residential lots	Townhouse	Apartments	(sqm)	(Boat s)
R1 - General Residential						
- Residential	52,591	15	54	162		
- Chapel / community centre	3,882					
- Neighbourhood shops @ Southern Foreshore Precinct					170	
R2 Low Density Residential						
-Residential	36,725	36				
Total Residential	89,316	51	54	162		
Total Retail					170	
SP3 Tourist - HOTEL	50,530					
- New building	50 500	40				
- Existing buildings	- 50,530	45				
Total tourist accommodation		85				
RE2 - Private Recreation	9,150					
E2 - Environmental Conservation	104,583					
RE1 - Public Recreation	94,709					
- Substation	7,400					
Total Public Open Space	208,442					
TOTAL	348,287					

Source: Mooney Mooney Table of Development - Urbis (update August 2021)

The above table shows that the development would provide 51 residential lots, 54 townhouses, 162 apartments as well as a hotel with 85 units.

The Southern Foreshore Precinct comprises 170 sqm retail space under the current planning proposal.

### **5** Traffic and Transport Assessment

This Section provides the results of the traffic and transport assessment undertaken for the development.

#### 5.1 Vehicle Trip Generation

#### 5.1.1 Trip Generation

Trip generation resulting from the potential rezoning and development of The Site was determined using the RMS Guide to Traffic Generating Developments (2002) and the RMS Technical Direction *2013/04a: Updated Traffic Surveys* (2013). A summary of the trip generation based on the Urbis concept plan is presented below in Table 5.1.

#### **Table 5.1: Trip Generation**

	A	M	P	ΡM	Sunda	iy Peak
Land Use	IN	OUT	IN	OUT	IN	OUT
Low Density Residential	20	73	78	21	20	73
Medium Density Residential	23	86	86	23	23	86
Retail	11	11	11	11	14	14
Hotel / Motel	7	28	28	7	7	28
Private Recreation	9	1	2	13	9	1
Total	70	199	205	75	73	202

As mentioned in Section 4, for the purpose of this assessment, the Private Recreation land use has been assumed to operate as a 60-berth dry stack dock. The future use of this land is subject to a further investigation and a separate planning proposal.

The analysis indicates that The Site could generate nearly 300 vehicle trips in the weekday and weekend peak hours. The key land use generator would be the proposed residential units and the Southern Foreshore Precinct including neighbourhood shops. The previous version of the Traffic and Transport Assessment considered a slightly higher development yield which led a higher trip generation of 294 vehicle trips during the AM peak hour and over 300 trips during the PM and weekend peak hours.

The modelling in this assessment has been maintained using the higher trip generation estimate as this gives a more conservative (worse case) assessment.

#### 5.1.2 Trip Assignment

The estimates of generated trips were distributed across the study area road network to understand the likely impacts on critical points of the road network. The distribution was based on the locality of sites shown on the concept plans and the following assumptions:

- 30 percent of generated trips will arrive from the north and 70 percent from the south, and this is based on JTW 2016 (refer Figure 3.2);
- 70 percent of generated trips will use the M1 Motorway to access The Site, considering that it's the major corridor carrying traffic north-south between major cities and suburbs;
- 30 percent of generated trips will use the Pacific Highway to access The Site, assuming it's used by local sites and neighbourhoods to access the development (such as Brooklyn); and

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 No assumptions were made for trip containment based on proposed local facilities. That is, all of the trips shown in Table 5.1 would be generated from areas external to the Site.

Refer to Appendix F for further details of the trip assignment.

#### 5.2 Impact on Public Transport

Refer to Figure 3.3 for an understanding of the travel mode profile generated under the proposed Concept Plan. The data indicates that public transport trips comprise of approximately 12 percent of all commuter trips and that these trips all occur by rail. A review of available capacity of rail services was undertaken to understand the potential impact on current peak hour commuter services by the proposal and is presented as an average peak hour load percentage against the total seating capacity of current services in 2014<sup>1</sup>:

- AM Peak: 64 percent (measured at Woy Woy); and
- PM Peak: 49 percent (measured at Hornsby)

The review indicated that existing rail services operating via Hawkesbury River station operate with some spare capacity that should easily accommodate any additional demand generated by the proposed development.

Increased patronage at Hawkesbury River station, as well as nearby major train stations with more frequent services (e.g. Berowra and Hornsby), as a result of the development will increase commuter parking requirements at those stations. This increase to commuter parking requirements would need to be addressed by Hornsby Shire Council / Central Coast Council and TfNSW. An allowance for these works could be included in a 7.11 Contribution which would apportion costs to the end land user rather than burdening the Authority undertaking the works.

Bus services are infrequent in nature due to the existing low demand. Less than one percent of trips are expected to use the bus as a travel mode, and The Site will therefore have no negative impact on bus services. Increase in bus routes and frequency will improve the public transport amenity of The Site and could be supported by developer contribution and this together with the planned increases in population are likely to attract a larger mode share in the future.

#### 5.3 Road Network Performance

This section of the report provides an understanding of the existing traffic conditions and potential traffic impact resulting from future rezoning and development of the area.

As mentioned in Section 3.8, two types of analysis were undertaken to assess the impact of the future rezoning and development on the road network performance, this included:

- Intersection analysis for three local intersections as highlighted in Figure 3.12. This analysis was undertaken using SIDRA Intersection 7; and
- Motorway segment analysis for M1 on and off ramps as highlighted in Figure 3.14. This analysis was undertaken using the Highway Capacity Manual 2010 methodology.

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<sup>&</sup>lt;sup>1</sup> Data taken from BTS Report on Train Statistics (December 2014)

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#### 5.3.1 Modelled Scenarios

The following scenarios have been modelled in SIDRA:

- Existing (2018) existing conditions only based on the traffic counts obtained for the project;
- Future year (2030) Without Development potential future conditions resulting from the projected traffic growth of the area without the development; and
- **Future year (2030) With Development** potential future conditions resulting from the rezoning proposal and development of the area in line with the Urbis concept plans presented in Appendix A.

To estimate the future scenario 2030 background traffic, a growth factor was applied to the existing intersection and permanent stations counts. The growth factor was derived from the STFM model flows provided by RMS (now TfNSW) for the area. Flows for the base year 2017, in addition to forecast flows for 2021, 2026, and 2031 were provided.

The following growth factors were used to factor the existing traffic counts of 2018:

- A growth factor of two percent per annum was applied to 2018 flows to estimate the 2021 flows;
- A growth factor of 0.6 percent per annum was applied to 2021 flows to estimate 2026 flows; and
- A growth factor 0.9 percent per annum was applied to 2026 flows to estimate the 2030 forecast flows.

#### 5.3.2 Intersection Analysis

#### 5.3.2.1 Traffic modelling performance measures

SIDRA Intersection 7 is a micro-analytical traffic modelling software tool that has been used to appraise the future conditions on the road network.

The 'Level of Service' (LoS) is the standard measure used to understand the operational performance of the network and intersections. This is defined as the qualitative assessment of the quantitative effect of factors such as speed, traffic volume, geometric features, delays and freedom of movement.

The LoS concept is applied to intersections through measures of effectiveness, as summarised in Table 5.2.

Intersection Control	Measure of Effectiveness
Priority controlled	Degree of Saturation
	Delay to critical movements (sec/vehicle)
	Queue length for critical movements
Traffic Signals	Average Delay (sec/vehicle)
	Delay to critical movements
	Degree of Saturation
	Cycle Length
	Queue length for critical movements
Roundabout	Average Delay (sec/vehicle)
	Delay to critical movements
	Degree of Saturation
	Queue length for critical movements

#### Table 5.2: Intersection Measures of Effectiveness

Source: RTA Guide to Traffic Generating Developments (2002)

The network conditions were evaluated using the LoS criteria defined in the *Guide to Traffic Generating Developments* prepared by NSW Roads and Maritime Services (now Transport for NSW). Details of the criteria are outlined in Table 5.3 below.

Level of Service	Average Delay per Vehicle (sec/veh)	Traffic Signals, Roundabout	Give Way & Stop Signs
Α	< 14	Good operation	Good operation
В	15 to 28	Good with acceptable delays & spare capacity	Acceptable delays & spare capacity
C	29 to 42	Satisfactory	Satisfactory, but accident study required
D	43 to 56	Operating near capacity	Near capacity & accident study required
E	57 to 70	At capacity; at signals, incidents will cause excessive delays Roundabouts require other control modes	At capacity, requires other control mode
F	> 70	Over capacity, unstable operation	Over capacity unstable operation

#### Table 5.3: Level of Service (LOS) Criteria for Intersections

Note:

1. The average delay assessed for signalised intersections is over all movements.

2. For roundabouts and priority control intersections (with Stop and Give Way signs or operating under the T-junction rule), the critical criterion for assessment is the movement with the highest average delay.

3. Average delay is expressed in seconds per vehicle.

#### 5.3.2.2 Modelled Network

The following intersections were modelled as part of the road network appraisal:

- 1. Old Pacific Highway/ B83 Pacific Highway operates as a give-way controlled T-intersection;
- 2. M1 Northbound ramps/ Peats Ferry Road/ B2 Site Road / Pacific Highway Link Road operates as a four-way roundabout; and
- 3. M1 Southbound ramps/ B83 Pacific Highway operates as an all way stopped controlled intersection under normal operations with the ability to operate under traffic signal control (temporary operations).

The modelled network is presented in Figure 3.12.

#### 5.3.2.3 Modelling Results

The results of the SIDRA intersection modelling for both existing and future development scenarios are presented in Table 5.4, Table 5.5 and Table 5.6 respectively and include weekday AM and PM peak hour assessments in addition to weekend. Detailed SIDRA analysis results are included in Appendix G.

		AM (Weekday)				PM (Weekday)			Sunday Peak		
Ref	Intersection	LOS	DoS	Max. Delay (s)	LOS	DoS	Max. Delay (s)	LOS	DoS	Max. Delay (s)	
1	Old Pacific Highway / Pacific Highway	А	0.051	6.3	А	0.067	6.4	А	0.216	7.9	
2	Old Pacific Highway / Peats Ferry Road	А	0.053	10.7	А	0.075	10.8	А	0.161	11.1	
3	Pacific Highway / M1 SB Ramps	В	0.164	16.7	В	0.228	14.7	В	0.540	19.5	

#### **Table 5.4: Intersection Operational Performance - Existing Conditions**

The results indicate that the existing network operates at the highest level of service with very low degrees of saturation at all intersections. This demonstrates that the existing road network contains a large amount of spare capacity.

#### Table 5.5: Intersection Operational Performance - Future year 2030 Without Development

		AM (Weekday)				PM (Weekday)			Sunday Peak		
Ref	Intersection	LOS	DoS	Max. Delay (s)	LOS	DoS	Max. Delay (s)	LOS	DoS	Max. Delay (s)	
1	Old Pacific Highway / Pacific Highway	А	0.059	6.4	А	0.077	6.5	А	0.257	8.4	
2	Old Pacific Highway / Peats Ferry Road	А	0.060	10.7	А	0.084	10.8	А	0.183	11.2	
3	Pacific Highway / M1 SB Ramps	В	0.185	17.1	В	0.258	15.3	В	0.611	22.4	

The results indicate that the existing road network and intersections perform well in the future year 2030 without the additional development trips leaving plenty of spare capacity.

#### Table 5.6: Intersection Operational Performance – Future year 2030 With Development

		AM (Weekday)		PM (Weekday)			Sunday Peak			
Ref	Intersection	LOS	DoS	Max. Delay (s)	LOS	DoS	Max. Delay (s)	LOS	DoS	Max. Delay (s)
1	Old Pacific Highway / Pacific Highway	А	0.186	7.5	А	0.192	7.4	А	0.444	11.3
2	Old Pacific Highway / Peats Ferry Road	А	0.095	10.9	А	0.166	11.1	А	0.262	11.9
3	Pacific Highway / M1 SB Ramps	В	0.337	15.0	В	0.340	16.2	В	0.663	24.0

The results indicate that the potential rezoning and development of The Site will not have a negative impact on the operation of the local road network in 2030 with the additional trips generated. All intersections continue to operate at satisfactory levels of service with spare capacity available.

#### 5.3.3 Motorway Segment Analysis (HCM Analysis)

#### 5.3.3.1 Link Analysis performance measures

The link analysis for all off ramp and on ramp segments was undertaken using the criteria detailed in the Highway Capacity Manual (HCM 2010) and *Austroads Guide to Traffic Management Part 3* section 4.4 and using the following assumptions:

- A conversion factor of two for both heavy vehicles and buses was adopted to convert the flows (in vehicles) to Passenger Car Units (PCUs); and
- A Peak Hour Factor (PHF) of 0.95 was used as worst-case scenario.

The performance of merge/diverge freeway segments are measured based on the densities of vehicles in the influence areas. Table 5.7 outlines the LoS criteria for varying traffic densities as adopted by the HCM 2010.

HCM 2010 also provides LoS criteria for standard freeway segments. Details are provided in Table 5.8.

#### Table 5.7: LoS Criteria for Freeway Merge and Diverge Segments

LOS	Density (pc/km/ln)	Comments
A	≤ 6	Unrestricted operations
В	> 6–12	Merging and diverging manoeuvres noticeable to drivers
С	> 12–17	Influence area speeds begin to decline
D	> 17–22	Influence area turbulence becomes intrusive
E	> 22	Turbulence felt by virtually all drivers
F	Demand exceeds capacity	Ramp and freeway queues form

Source: Exhibit 13-2 in the HCM 2010

#### Table 5.8: LoS Criteria for Basic Freeway Segments

Oritoria		LOS							
Criteria	Α	В	С	D	E				
	FFS = 120 k	m/h							
Maximum density (pc/km/ln)	7	11	16	22	28				
Minimum speed (km/h)	120.0	120.0	114.6	99.6	85.7				
Maximum (v/c)	0.35	0.55	0.77	0.92	1.00				
Maximum service flow rate (pc/h/ln)	840	1320	1840	2200	2400				
	FFS = 110 k	m/h							
Maximum density (pc/km/ln)	7	11	16	22	28				
Minimum speed (km/h)	110.0	110.0	108.5	97.2	83.9				
Maximum (v/c)	0.33	0.51	0.74	0.91	1.00				
Maximum service flow rate (pc/h/ln)	770	1210	1740	2135	2350				
	FFS = 100 k	m/h							
Maximum density (pc/km/ln)	7	11	16	22	28				
Minimum speed (km/h)	100.0	100.0	100.0	93.8	82.1				
Maximum (v/c)	0.30	0.48	0.70	0.90	1.00				
Maximum service flow rate (pc/h/ln)	700	1100	1600	2065	2300				
	FFS = 90 kr	n/h			•				
Maximum density (pc/km/ln)	7	11	16	22	28				
Minimum speed (km/h)	90.0	90.0	90.0	89.1	80.4				
Maximum (v/c)	0.28	0.44	0.64	0.87	1.00				
Maximum service flow rate (pc/h/ln)	630	990	1440	1955	2250				

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Source: Exhibit 23-2 in the HCM 2000; the content here is consistent with the LoS criteria in the HCM 2010 (Exhibit 11-5)

The approximate capacity of the entry and exit ramps as well as the freeway segments upstream and downstream of the ramps were calculated using the HCM 2010 requirements. The criteria are provided in Tables 5.9 to 5.11.

Table 5.9: Approximate C	Capacity o	of Ramp	Roadways	in Passen	der cars/hour
	apaony o		nounayo		goi ouio/iioui

Free flow encod of some SED (loss /b)	Capacity (pc/h) <sup>(1)</sup>					
Free-flow speed of ramp, SFR (km/h)	Single-lane ramps	Two-lane ramps				
> 80	2200	4400				
> 65–80	2100	4200				
> 50-65	2000	4000				
≥ 30–50	1900	3800				
< 30	1800	3600				

Source: Exhibit 13-10 in the HCM 2010

Table 5.10: Capacity Value	s for Merge Areas ir	Passenger cars/hour
----------------------------	----------------------	---------------------

	Capacity of u	(pc/h) <sup>(1)(3)</sup>	Max desirable	Max desirable			
Freeway free- flow speed		flow entering	flow entering influence				
(km/h)	2	3 4 > 4		> 4	merge influence area V <sub>R12</sub> (pc/h) <sup>(2)</sup>	diverge area V <sub>12</sub> (pc/h) <sup>(2)</sup>	
120	4800	7200	9600	2400/ln	4600	4400	
110	4700	7050	9400	2350/ln	4600	4400	
100	4600	6900	9200	2300/ln	4600	4400	
90	4500	6750	9000	2250/ln	4600	4400	

Source: Exhibit 13-8 in the HCM 2010

#### Table 5.11: Capacity Values for Diverge Areas in Passenger cars/hour

	Maximum U	pstream, v <sub>FI</sub> or l (p			
Freeway Free-Flow Speed (km/h)	N	umber of Lanes	Max Flow Entering Influence Area, v <sub>12</sub> (pc/h)		
	2	3	4	> 4	
120	4800	7200	9600	2400/In	4400
110	4700	7050	9400	2350/In	4400
100	4600	6900	9200	2300/In	4400
90	4500	6750	9000	2250/In	4400

Source: Exhibit 25-14 in the HCM 2010

#### 5.3.3.2 Analysed Links

As shown in Figure 3.14, the following M1 off-ramp diverge segments and on-ramp merge segments have been assessed using the HCM 2000 criteria:

- 1. Pacific Highway (M1) Off-Ramp Diverge (Northbound);
- 2. Pacific Highway (M1) Off-Ramp Diverge (Southbound);
- 3. Pacific Highway (M1) On-Ramp Merge (Northbound); and
- 4. Pacific Highway (M1) On-Ramp Merge (Southbound).

#### 5.3.3.3 Analysis Results

The results of the link analysis for both existing and 2030 future development scenarios are presented in Table 5.12, Table 5.13 and Table 5.14 respectively and include weekday AM and PM and weekend peak hour assessments. Detailed analysis results are provided in Appendix H.

#### Table 5.12: Link Operational Performance – Existing Conditions

No	Peak	Freeway- Ramp	Freeway before ramp - All Lanes			Ra	amp - All Lan	es	Freeway after ramp - All Lanes		
	Hour	Terminal Segment	V/C	Density (pc/km/ln)	LoS	V/C	Density (pc/km/ln)	LoS	V/C	Density (pc/km/ln)	LoS
1		Pacific Highway (M1) Off-Ramp (Northbound)	0.29	8.86	В	0.04	1.28	А	0.28	5.94	А
2	AM	Pacific Highway (M1) Off-Ramp (Southbound)	0.77	16.37	С	0.04	1.35	А	0.76	16.18	С
3	Alvi	Pacific Highway (M1) On-Ramp (Northbound)	0.28	5.96	А	0.02	0.72	А	0.29	7.26	В
4		Pacific Highway (M1) On-Ramp (Southbound)	0.79	16.80	С	0.04	1.47	А	0.80	18.14	D
1		Pacific Highway (M1) Off-Ramp (Northbound)	0.73	16.00	С	0.05	1.81	А	0.71	15.23	С
2		Pacific Highway (M1) Off-Ramp (Southbound)	0.37	10.52	В	0.02	0.82	А	0.37	7.81	В
3	PM	Pacific Highway (M1) On-Ramp (Northbound)	0.66	14.17	С	0.06	2.14	А	0.68	15.34	С
4		Pacific Highway (M1) On-Ramp (Southbound)	0.37	7.93	В	0.04	1.42	А	0.38	9.89	В
1		Pacific Highway (M1) Off-Ramp (Northbound)	0.54	13.81	С	0.12	3.93	А	0.51	10.90	В
2	Sunday	Pacific Highway (M1) Off-Ramp (Southbound)	0.56	13.97	С	0.09	3.02	А	0.53	11.43	В
3	Sunday	Pacific Highway (M1) On-Ramp (Northbound)	0.44	9.40	В	0.07	2.35	А	0.46	10.93	В
4		Pacific Highway (M1) On-Ramp (Southbound)	0.55	11.66	В	0.07	2.21	А	0.56	13.57	С

			Freew	vay before ran All Lanes	np -	Rar	np - All Lar	ies	Freew	Freeway after ramp - All Lanes		
No	Peak Hour	Freeway- Ramp Terminal	V/C	Density (pc/km/ln)	LoS	V/C	Density (pc/km/l n)	LoS	V/C	Density (pc/km/ln)	LoS	
1		Pacific Highway (M1) Off- Ramp (Northbound)	0.27	8.37	В	0.04	1.46	А	0.25	5.41	А	
2	AM	Pacific Highway (M1) Off- Ramp (Southbound)	0.75	16.30	С	0.06	2.05	А	0.74	15.75	С	
3	AIVI	Pacific Highway (M1) On- Ramp (Northbound)	0.32	6.73	В	0.02	0.82	А	0.32	8.04	В	
4		Pacific Highway (M1) On- Ramp (Southbound)	0.71	15.21	С	0.05	1.67	А	0.73	16.72	С	
1		Pacific Highway (M1) Off- Ramp (Northbound)	0.75	16.25	С	0.06	2.04	А	0.73	15.65	С	
2	PM	Pacific Highway (M1) Off- Ramp (Southbound)	0.37	10.55	В	0.03	0.93	А	0.37	7.81	В	
3	FIVI	Pacific Highway (M1) On- Ramp (Northbound)	0.75	16.05	С	0.07	2.44	А	0.77	17.19	D	
4		Pacific Highway (M1) On- Ramp (Southbound)	0.42	8.99	В	0.05	1.51	А	0.43	10.89	В	
1		Pacific Highway (M1) Off- Ramp (Northbound)	0.59	14.59	С	0.13	4.46	А	0.55	11.84	В	
2	Sunday	Pacific Highway (M1) Off- Ramp (Southbound)	0.60	14.63	С	0.10	3.35	А	0.58	12.29	С	
3	Sunday	Pacific Highway (M1) On- Ramp (Northbound)	0.58	12.34	С	0.08	2.65	А	0.60	13.77	С	
4		Pacific Highway (M1) On- Ramp (Southbound)	0.61	12.96	С	0.09	3.04	А	0.63	15.00	С	

#### Table 5.13: Link Operational Performance – Future year 2030 Without Development

				way before ra All Lanes			Ramp - All Lanes			way after rar All Lanes	np -
No.	Peak Hour			Density (pc/km/ln)	LoS	V/C	Density (pc/km/ln)	LoS	V/C	Density (pc/km/ln)	LoS
1		Pacific Highway (M1) Off- Ramp (Northbound)	0.27	8.58	В	0.07	2.23	А	0.25	5.41	А
2	AM	Pacific Highway (M1) Off- Ramp (Southbound)	0.76	16.31	С	0.06	1.86	А	0.74	15.84	С
3	AIVI	Pacific Highway (M1) On- Ramp (Northbound)	0.32	6.89	В	0.05	1.58	А	0.34	8.37	В
4		Pacific Highway (M1) On- Ramp (Southbound)	0.91	19.36	D	0.10	3.42	А	0.94	21.05	D
1		Pacific Highway (M1) Off- Ramp (Northbound)	0.75	16.29	С	0.07	2.23	А	0.73	15.65	С
2	PM	Pacific Highway (M1) Off- Ramp (Southbound)	0.37	10.63	В	0.05	1.70	А	0.36	7.67	В
3	1 101	Pacific Highway (M1) On- Ramp (Northbound)	0.75	16.12	С	0.08	2.82	А	0.78	17.36	D
4		Pacific Highway (M1) On- Ramp (Southbound)	0.43	9.15	В	0.07	2.40	А	0.45	11.28	В
1		Pacific Highway (M1) Off- Ramp (Northbound)	0.60	14.77	С	0.16	5.25	А	0.55	11.85	В
2	Sunday	Pacific Highway (M1) Off- Ramp (Southbound)	0.61	14.65	С	0.09	3.16	А	0.58	12.39	С
3	Sunday	Pacific Highway (M1) On- Ramp (Northbound)	0.51	10.79	В	0.10	3.40	А	0.53	12.52	С
4		Pacific Highway (M1) On- Ramp (Southbound)	0.63	13.53	С	0.13	4.28	А	0.67	15.87	С

#### Table 5.14: Link Operational Performance – Future year 2030 With Development

The following can be deduced from the above tables:

- 1. All on/off ramps are performing at LoS of A or B in the existing, with development and without development scenario for both AM and PM peaks of a weekday and the peak hour in the weekend. No mitigations are required; and
- 2. All motorway segments before and after the ramp (i.e M1 upstream and downstream segments) are operating with a LoS of D or better for all scenarios. It should be noted that the LoS D is a result of background traffic growth on M1 rather than a result of the development.

#### 5.4 Parking Requirement and Provision

Parking requirements for the development were estimated based on rates outlined in the Gosford DCP and the Guide to Traffic Generating Developments (RMS, 2002). The parking rates adopted for the development are outlined in Table 5.15 below.

#### Table 5.15: Parking Rates

Land Use	Parking Rate	Units/Remarks	Source
Low Density Residential	2	space per dwelling	Gosford DCP Rates 2013
Medium Density Residential	1.5	space/dwelling + 0.2 space per dwelling for visitor	-
Retail (Local Centre)	0.033	1 space per 30 sq.m	
Hotel	1	space per room	-
Dry stack boat storage (indicative only)*	0.2	Spaces per Berth	Guide to Traffic Generating Developments, 2002

\*The dry stack boat storage is subject to separate planning proposal process as described in Section 1.1.

The application of the parking rates in Table 5.15 to the proposed development land uses shown in Table 5.16 results in the parking requirements shown in Table 5.17.

A plan showing the position of locations 1-7 is presented in Figure 5.1.

#### Table 5.16: Summary of Land Use Plan Based on Locations

Land Us	e	Locatio n 1	Locatio n 2	Locatio n 3	Locatio n 4	Locatio n 5	Locatio n 6	Locatio n 7	Total	Unit
Low Residenti	Density ial	54	35	7	-	9	-	-	105	dwellings
Medium Residenti	Density ial	-	47	57	58	-	-	-	162	apartment s
Retail		-	-	170	-	-	-	-	170	Sq.m
Hotel		-	-	-	-	-	85	-	85	rooms
Dry-stack storage (indicative		-	-	-	-	-	-	60	60	slots

#### Table 5.17: Minimum Parking Requirements based on Concept Plan

Land Use		Location 1	Location 2	Location 3	Location 4	Location 5	Location 6	Location 7	Total
Low Residential	Density	108	70	14	-	18	-	-	214
Medium Residential	Density	-	80	97	99	-	-	-	276
Retail		-	-	6	-	-	-	-	6
Hotel		-	-	-	-	-	85	-	85
Total Demand	Parking	108	150	117	99	18	85	-	577
Dry-stack bo (indicative or								12	

\*The dry stack boat storage is subject to separate planning proposal process as described in Section 1.1.

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Figure 5.1: Proposed Development Zoning Map



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Table 5.17 indicates that a minimum 577 parking spaces would be required for the development of which 486 spaces would be for residential uses and 85 for the hotel. A minimum of six parking spaces would be required for convenience shops located within the Southern Foreshore Precinct.

The proposed parking provisions for the development are provided in Table 5.18. The development would include a minimum of 585 parking spaces (without the dry-stack boat storage area) and therefore an adequate number of spaces will be provided to meet the expected parking demand.

Land Use	Location 1	Location 2	Location 3 <sup>**</sup>	Location 4	Location 5	Location 6	Location 7
Low Der Residential	nsity 108	74	14	-	18	-	-
Medium Der Residential	nsity -	116#	128#	127*	-	-	-
Retail	-	-	**	-	-	-	-
Hotel	-	-	-	-	-	*	-
Dry stack storage	boat						97
Total Parl Provision	king 108	190	142	127	18	0	(97)

#### Table 5.18: Parking Provision within the Concept Plan

\* The hotel parking requirement (location 6) is provided within the at-grade car park adjacent to the buildings at location 4

\*\* 6 parking spaces for retail are includinged within location 3

<sup>#</sup> Basement Car Park for apartments (buildings)

Parking spaces for medium density residential apartments in Locations 2, 3 and 4 would be provided within basement carparks. The hotel parking would be located as an at-grade dedicated carpark at Location 4. It is recommended that a secured car park area is provided for the hotel.

A dedicated parking area for the dry-stack boat storage areas would be provided at Location 7 (shown indicatively only as the dry-stack boat storage in the concept plan). This area will be subject to a different development proposal process.

## 6 Pedestrian and Cycling Strategy

This section highlights the pedestrian and cycling opportunities within the Concept Plan.

As discussed in Section 3.7, there are currently a limited number of footpaths and bicycle facilities within The Site area and as a result, current opportunities for cycling are limited. Figure 6.1 identifies opportunities for both cyclists and pedestrians within and around the development. Figure 6.2 shows the detail of the proposed pedestrian connection and the proposed cross section of the dedicated cycle lanes.

Figure 6.1: Proposed Pedestrian and Cycling Strategy



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#### Figure 6.2: Proposed infrastructure around pedestrian connection (left) and concept cross section for dedicated cycle lanes

1.8 m 1m

The following improvements/enhancements are recommended to provide better pedestrian and cycling access:

- A dedicated cycle lane can be provided along the Old Pacific Highway to tie-in with 'on road' cycle route already present. Improvements might be considered for the existing cycle lane on Old Pacific Highway with recommendations to extend the cycle lane and formalise it to connect Mooney Mooney area with Hawkesbury River Station in Brooklyn.
- There is an opportunity for providing a number of bicycle parking spaces at Hawkesbury River Station to promote active transport.
- Gosford Council DCP rates (2013) do not specify any rates for cycle parking provision, however, it is
  recommended that the developer provides a number of cycle parking spaces for the residents of the
  development within the buildings to promote active transport.
- A shared pedestrian/cycling path is recommended along the Pacific Highway Link through the underpass. The lane continues north with a crossing provided on the Old Pacific Highway and an option for cyclists to travel southbound using the cycle only lane or northbound using the provided shared path that extends into the north-east corner of the development and into Point Road.
- Maintaining the existing pedestrian underpass crossing M1 and links both eastern and western areas of the development and extending it to serve the localities of the proposed development (pink dotted line in Figure 6.1).
- Providing a suitable cycling and pedestrian crossing on the Old Pacific Highway as indicated in Figure 6.1. This provides an opportunity to improve the bus stops on the Old Pacific Highway with possibilities to provide bus shelters by re-configuring curb lines.
- Reduction of the posted speed on the Old Pacific Highway between the Highway Pacific Link and Point Road from 60kph to 50kph is highly recommended as a traffic calming measure to create a safer environment for both pedestrians and cyclists.
- Access to Peat Island is limited to a narrow road that runs along the causeway from the western side of the Mooney Mooney peninsula to Peat Island. The causeway itself is approximately 200m in length and 5m wide (3.0-3.5m wide road carriageway and approximately 1.5m wide footpath = 5m road reserve) between fences, which limits the capacity of the link and access to Peat Island. Due to this constraint and investigations indicating that widening is not feasible from an economic, environmental or heritage perspective, vehicular traffic operating across the causeway is recommended to be limited to service vehicles only and other authorised vehicles.
- A shuttle bus service could be provided for the hotel employees and visitors and that would be operating between the hotel dedicated parking area at location 4 and the hotel.

## 7 Transport Management and Access Strategy

This section provides recommendations for enhancing the public transport facilities and improving accessibility within the development.

#### 7.1 Transport Management

Around 12% of the local population utilise trains as their mode of travel, however this percentage could be increased by implementing the following measures:

- Improving the connectivity between the Hawkesbury River Train Station and the development by improving the cycling lanes and extending them to link to the residential and leisure uses within the development;
- Provision of secured cycle parking facilities at the station and within the development to promote cycling;
- Enhancing the car parking areas at the station and providing park and ride facilities; and
- Provision of facilities for people with DDA needs should be considered as the station is not DDA compliant.

Statistics show that less than one percent of the local population travel by bus. This percentage could be increased with the following considerations:

- Improving 592 bus services by providing more frequent services and operating during off-peaks, Sundays, and public holidays;
- Providing frequent services to Peat Island as it's currently serviced by one trip a day; and
- Improving the bus stops on the Old Pacific Highway with possibility to provide bus shelters. Refer to Figure 6.1.

For active transport refer to Section 6 – Pedestrian and Cycling Strategy.

#### 7.2 Access Strategy

#### 7.2.1 Access and Parking

Vehicular access to Peat Island will be limited to service vehicles and authorised vehicles with recommendation to provide a shuttle bus service to transfer hotel visitors and employees between the island and the hotel designated parking area. Pedestrians and cyclists will be able to access the island as shown in Figure 6.1.

Parking for Peat Island will be accommodated on the Mooney Mooney Peninsula in the underground car park at Location 4. A secured parking area should be designated for the hotel use.

Adequate parking for all land uses identified in the Concept Plan will be provided in accordance with the DCP as part of future DAs. Refer to section 5.4 for further information on parking provision and proposed locations.

Further consultation with TfNSW as part of future development applications and the detailed design stages will be used to determine an appropriate design solution for The Site.

## 8 Local Area Traffic Management Strategy

#### 8.1 Posted Speed Reduction

The current posted speed limit on the B83 Pacific Highway and the Pacific Highway Link Road is 60 km/h and it is assumed that all other local roads in The Site area are 50 km/h, given their residential or low-density nature. The concept plan aims to reduce all local road posted traffic speeds limits within The Site to 50 km/h, which would support the proposed future uses of this area and the characteristics of its residents. This 50 km/h urban speed environment would cover the following roads:

- Peats Ferry Road;
- Peat Island Road;
- Pacific Highway Link Road;
- Kowan Street; and
- Chapel Road.
- The Pacific Highway between the Pacific Highway Link Road and Point Road.

Along with setting the speed environment the local road network will be planned so that safe and efficient movement is promoted. This will be achieved through aligning intersections and access points to help to minimise conflict and promote safe and efficient movement.

#### 8.2 Traffic Calming Measures

In addition to speed reduction, there are other measures that could be considered for traffic calming particularly around the pedestrian/cycling crossing on the Old Pacific Highway and the residential areas of the development. Figure 8.1 below highlights potential locations within the site that could benefit from the provision of traffic calming measures.

There are different traffic calming measures that could be implemented within the development, some of which are detailed below as stated in Section 6.6.7 of the "Global Street Design Guide 2016" by Global Designing Cities Initiative and NACTO (National Association of City Transportation Officials).



Figure 8.1: Proposed Locations for Local Area Traffic Management

#### 8.2.1 Lane Narrowing

Narrow lanes as shown in Figure 8.2 can be used within the residential areas and those reduce the rightof-way and make drivers wary of the traffic and adjacent users. It provides opportunities for using the additional space for pedestrians or cycling facilities

#### Figure 8.2: Lane Narrowing Measure



Source: Global Street Design Guide

#### 8.2.2 Gateway Treatments

Gateway treatments alert drivers that they are entering a slower area. This measure may include signage, speed tables, raised crossings, and curb extensions. Figure 8.3 provides an example of gateway treatments.

#### Figure 8.3: Gateway Treatments



Source: Global Street Design Guide

#### 8.2.3 Medians and Refuge Islands

Raised centre medians and pedestrian refuge islands as shown in Figure 8.4 can be used to reduce lane width for vehicles and provide safe linkage for pedestrians.

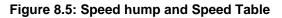
Figure 8.4: Median and Refuge Island

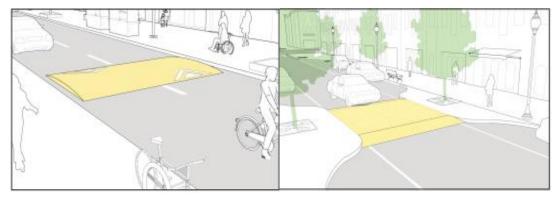


Source: Global Street Design Guide

#### 8.2.4 Speed humps and Speed Tables

Speed humps and tables are effective traffic calming measures to reduce the speed of vehicles on the road. A speed table can be converted into a raised crossing by providing a pedestrian crossing on its flat top.





Source: Global Street Design Guide

## 9 Wayfinding Strategy

A desktop review of the existing signage has been undertaken for the Mooney Mooney area for a radius of 2 km. The existing sign faces have been captured along M1 and Pacific Highway for accessing and exiting The Site. The inventory of existing signs is provided in Appendix I.

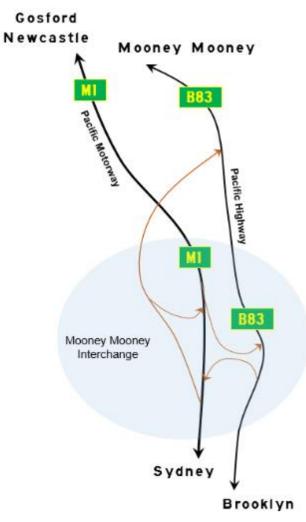
#### 9.1 State Directional Signs

Considering that the proposed development will not introduce any changes to the existing road network and will not include major destinations that commuters travelling along M1 and the Pacific Highway are to be informed of, it is not anticipated that changes will be required for the existing state green signs or that there will be a need to install new wayfinding signs.

The existing focal point map shown in Figure 9.1 below will be valid after the completion of the development. The map shows that Mooney Mooney Interchange has been adequately signposted in accordance with the TfNSW standards of guide signposting.

Additionally, the M1 on and off ramps and the associated intersections 2 and 3 have appropriate advance direction and intersection direction signs. The reassurance direction signs have also been provided after the intersections. Along the M1, the sign for the rest area and stopping bays have also been provided in both directions.

Figure 9.1: Existing Focal Point Map



#### 9.2 Service Signs

With the introduction of the proposed land use such as the Peat Island Hotel, it is suggested that information about these facilities be provided to the commuters, in the form of service signs (blue).

The suggested changes to the existing signs are as detailed below:

• The sign 'M1\_NB\_02" located 1.2km south of northbound off-ramp could be updated to show that a hotel and retail facilities are located at Mooney Mooney. The proposed sign is shown in Figure 9.2.

#### Figure 9.2: Existing and Proposed Sign Face of M1\_NB\_02 Sign

Existing (M1\_NB\_02)

Proposed



• The sign 'M1\_SB\_04" located 300m north of southbound off-ramp could be modified to show the service symbols as shown in Figure 9.3 below.

#### Figure 9.3: Existing and Proposed Sign Face of M1\_SB\_04 Sign

Existing (M1\_SB\_04)



Proposed



- The ambulance service sign to be removed underneath the sign MMPI\_04. Another ambulance service sign underneath the sign MMPI\_10 to be re-oriented on the same island to show the new location of the ambulance service station which will be accessible via the south approach of the Pacific Highway/Old Pacific Highway intersection. Another ambulance service sign MMPI\_23 to be relocated to the new entrance of the station instead of an intersection direction sign.
- Provision of signs indicating locations of public parking areas within the area.
- The cycle land sign should be used to designate an exclusive bicycle lane where needed.
- A shared path sign should be used to designate a path that is shared by cyclists and pedestrians.
- Local Area Traffic Management (LATM) signs are to be used in reduced speed areas, pedestrian and cycle crossing areas, and within residential areas and those should include regulatory and warning signs.

Sign face requirements and location of signs should be identified through a detailed wayfinding strategy.

## **10 Key Findings**

The key findings of the assessment include the following:

- The existing road network contains spare capacity to accommodate the additional trips associated with proposed rezoning of the area.
- Consultation with TfNSW was important for the masterplan layout, allowing the proposal to be refined and address issues raised.
- The expected development resulting from the rezoning proposal is not expected to have a negative
  impact on the operational performance of the local road network. The modelling results for both the
  existing and future 2030 scenario indicate that the road network still contains spare capacity after full
  development of The Site and that all key intersections operate at satisfactory Level of Service (LoS).
- All motorway segments including the on and off ramps of the M1 are operating at an acceptable LoS and will continue to do so under the future 2030 scenario after the completion of the development.
- Parking provision has been assessed and is found to be adequate for the development.
- The causeway to Peat Island is narrow and is considered suitable for pedestrian, cyclist and one-way vehicular access only. It is therefore recommended that the vehicular movements are limited to service vehicles and shuttle services to and from the hotel.
- The additional development traffic that is expected to travel south on the Hawkesbury River Bridge is estimated to be around 60 trips in the AM, PM, and Sunday peak respectively and therefore will not have any negative impact on the Bridge operation at any time.

The major recommendations related to the public transport are as follows:

- Improving the connectivity between the Hawkesbury River Train Station and the development by improving the cycling lanes and extending them to link to the residential and leisure uses within the development.
- Commuter car parking requirements at Hawkesbury River Train Station, along with Berowra and Hornsby Train Stations, are expected to increase as a result of the development. It is recommended that a Section 7.11 Contribution be considered to accommodate any costs associated with works to improve parking provision at the relevant stations.
- Possibility of providing several secured cycle parking facilities at the station and within the residential buildings to promote cycling.
- Opportunity for providing park and ride facilities at the station.
- Provision of facilities for people with DDA needs could be considered as the station is not DDA compliant.
- Improving 592 bus services by providing more frequent services and operating during off-peaks, Sundays, and public holidays.
- Providing frequent services to Peat Island as it's currently serviced by one trip a day.
- Improving the bus stops on the Old Pacific Highway with possibility to provide bus shelters and improve the bus stop facilities.

The major recommendations related to the active transport are as follows:

Gosford Council DCP (2013) do not specify any rates for cycle parking provision, however, it is
recommended that the developer provides a number of cycle parking spaces at a rate of one bike
parking per residential unit for the residents of the area as an attempt to promote active transport.

370106 | T&T | L | 9 August 2021

https://mottmac.sharepoint.com/teams/pj-c1752/do/Develop/Transport/9. Update Aug 2021/Report/MMPI-TrafficTransport Assessment - Aug 2021 RevL.docx

- Retain the existing shared pedestrian/cycling path along the foreshore on the western side of the development.
- A shared pedestrian/cycling path can be provided along the Pacific Highway Link through the underpass. The lane continues north with a crossing provided on the Old Pacific Highway and an option for cyclists to travel southbound using the cycle only lane or northbound using the provided shared path that extends into the north-east corner of the development and into Point Road.
- Improvements to the existing pedestrian underpass crossing M1 and links both eastern and western areas of the development and extending it to serve the localities of the proposed development.
- Providing a suitable cycling and pedestrian crossing on the Old Pacific Highway as indicated in Figure 6.1. This provides an opportunity to improve the bus stops on the Old Pacific Highway with possibilities to provide bus shelters by re-configuring curb lines.
- Reduction of the posted speed on the Old Pacific Highway between the Highway Pacific Link and Point Road is highly recommended as a traffic calming measure to create a safer environment for both pedestrians and cyclists.
- Access to Peat Island is limited to a narrow road that runs along the causeway from the western side of the Mooney Mooney peninsula to Peat Island. The causeway itself is approximately 200m in length and 5m wide (3.0-3.5m wide road carriageway and approximately 1.5m wide footpath) between fences. This limits the capacity of the link and access to Peat Island. Due to this constraint and investigations indicating that widening is not feasible from an economic, environmental or heritage perspective, vehicular traffic operating across the causeway is limited to service vehicles only and other authorised vehicles.

## Appendices

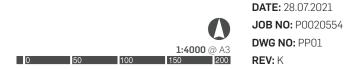
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## A. Concept Plan





### **MOONEY MOONEY & PEAT ISLAND FINAL CONCEPT PLAN**



## **B.** Previously raised and addressed planning and traffic Issues

# Previous Department of Planning and Environment queries with responses as detailed in the report issued 12<sup>th</sup> September 2016

Issues	Responses (as raised and discussed in the report submitted on 12 <sup>th</sup> September 2016)
Strategic suitability of The Site	The planning proposal is demonstrated to align with the intention of the Draft Central Coast Regional Plan and support future growth near centres and along established transport corridors – refer to section 2 and Figure 2.1. The planning proposal is identified to support the goals and targets identified in NSW 2021 – refer to section 2
Consultation with RMS	Refer to the below for responses to RMS issues listed in the Department of Planning and Environment letter dated 22/9/2015 (ref 15/13626) and the following section of Appendix B
Suitability of development intensity	The capacity appraisal of the existing road network demonstrates that there is adequate spare capacity across the existing local road network to support future increases in development. The appraisal also highlights that the traffic generation from the proposed concept would be moderate and can be managed under current arrangements – refer to section 5
Access to island	Access to the island will be managed and limited to service vehicles and authorised vehicles only. This will ensure that the limited operating capacity and potential for conflict with pedestrians can be appropriately controlled – refer to sections 6 and 7.
Access to the network and safety	The assessment has indicated that the development proposal will have a minor impact on network operations or current historical road safety trends – refer to section 3.9, 5, 6, and 7.
Services centre and rest area	The potential development of a service station will assist in the provision of roadside services for northbound traffic and help to manage historical road safety issues and is expected to be supported by improvements in wayfinding – refer to section 7.2 and Appendix I
Managing potential conflict with existing recreation facilities	The development proposal has limited impact on current recreation activity and proposes an upgrade and expansion to support future needs – refer to Figure 1.1 and section 5.1.
Public transport access	The proposed development aligns with existing service routes and stops, and it is acknowledged through growth along the corridor that there would be potential for future increases in service frequency as part of Government's progressive improvement to public transport services – refer to section 5 and 7.
Connectivity with local facilities and services	Established roads and services, such as the Old Pacific Highway offer efficient and safe connections to surrounding facilities and services located in Brooklyn and Berowra, and more regional based facilities in Gosford and Hornsby. These can be accessed in some cases by both active and public transport - refer to sections 2, 6,7, 8, and 9.
Consultation with TfNSW	Refer to the below for responses to TfNSW issues listed in the Department of Planning and Environment letter dated 22/9/2015 (ref 15/13626)
Public transport demand and servicing	There is adequate capacity on current services to accommodate future increases in demand both by rail and bus. It would be desirable to increase bus route service frequency and consider commuter parking at the rail station to make this a more desirable method of travel - refer to section 3, 5.2, and 7.
Active transport opportunities (TDM)	Active transport is currently supported by limited footpath and cycle lane provision and the development will consider significant internal improvements to encourage active transport activity and investigate future upgrades to improve connectivity to surrounding centres - refer to section 6.

## Previous issues raised by TfNSW and RMS during the consultations with them and discussed in the report submitted on 12<sup>th</sup> September 2016

Agency	Contact Name, Role/Title	Issues raised	Resolution/Action
RMS Hunter Region	Adam Thomas Leader – Network Optimisation	Need to consider safety and crash trends wayfinding needs and avoid modifying motorway ramps.	, Crash data provided by RMS and appraised as part of the study - refer to <b>section 3.9</b> and <b>Appendix E</b> Impact on motorway ramps and wayfinding have been assessed – refer to <b>section 5.3.3</b> and <b>Appendix H</b> .
RMS Hunter Region	<b>Ken Saxby</b> Network & Safety Manager – Network Optimisation	Consider B-double permissible routes, proposed RMS works in the vicinity of The Site and road names.	This has been tested initially in the 2016 ereport, however, since then, plans to develop the B2 site as a petrol filling station has been cancelled and therefore the development access assessment by B-double is not required. No proposed RMS works proposed and road names adjusted to align with RMS feedback.
RMS Hunter Region	Paula Goodwin Senior Property Officer	Raised safety issues related to the M1, boat ramp and public toilets area.	Road safety issues considered as part of the appraisal process and the proposal is considered not to impact on these existing features - refer to <b>sections 3.9 and 5</b> .
RMS TMC Sydney	<b>David Wainwright</b> Principal Manager – Traffic Operations	may not be sufficient to capture the	sHCM analysis has been undertaken to assess the performance of all motorway segments cincluding on and off- ramps of the M1 – refer to section <b>5 and Appendix H.</b>
TfNSW Active Transport	<b>Duncan Tjin</b> Senior Transport Planner – Active Transport	TfNSW satisfied with concept provision of walking and cycling connections to bus stops. Suggested to explore cycle link to Hawkesbury River rail station.	Cycle link to Hawkesbury River rail station to be discussed with RMS as part of future planning proposals. TfNSW indicated walking and cycling connectivity should be delivered by developers, which is acknowledged and addressed in <b>section 6</b> .
TfNSW Bus Planning	Gordon Hunt Service Planner – Service Planning and Development, Infrastructure and Services	Transdev bus route 592 serves the Mooney Mooney area Services are limited, and the developmen should support the potential for more frequent and weekend services.	Possible development funding to support improved service provision is supported and twill be investigated as part of subsequent studies. The possibility of diverting the 592 services to Peat Island Road and deemed not to be required - refer to <b>section 6 and 7</b> .

## C. Other raised issues by Council, TfNSW, and other agencies

### PP studies which request more technical studies

Submitted report	What is required	Response
Annexure M - Traffic & Transport Review By Mott Macdonald	transport management and access strategy that would facilitate and support safe and efficient access by all modes of travel to and from the site	Refer to Section 6, 7
M - Traffic & Transport Review By Mott Macdonald	wayfinding strategy for the area and to address access needs from the strategic transport network	Refer to Section 9
M - Traffic & Transport Review By Mott Macdonald	local area traffic management strategy that would support safe movement along the local network and improve connectivity to surrounding centres	Refer to Section 8
M - Traffic & Transport Review By Mott Macdonald	more detailed traffic and transport assessment to support future DA submissions and its potential staging and investigate possible future upgrades regional links and services	Refer to <b>Section 3,4,5,6,7,8,9</b>
M - Traffic & Transport Review By Mott Macdonald	Crime Prevention through Environmental Design (CPTED) assessment	DA Stage

## Issues raised by the Council

Council staff requirements	Further studies and key issues	Response
Steve Green - Transport	There are currently a limited number of footpaths and bicycle facilities within The Site area. There are no commitments to improve cycle routes or other facilities in the Mooney Mooney area.	A cycling and parking strategy has been developed for the site with proposed possible improvements to the facilities at the Hawksbury River Train station and the bus stops on the Old Pacific Highway. Refer sections <b>5,6,7,8</b>
	I would question the comments claimed that the PP is located in a location where current education facilities and services like primary and high schools and TAFEs are actually at a considerable distance from proposed residences. Nearest Primary School would be Brooklyn which is 4.7km away. Nearest High school could be Gosford or Hornsby. The nearest TAFE would probably be located in Gosford or Hornsby. Nearest University would probably be Newcastle University through the Central Coast Campus or Sydney University.	
	For cycling to be viable a 5-kilometre cycling distance is favourable provided a safe route can be developed. One of the key actions is to investigate is improved east/west pedestrian/cyclist connectivity from Peat Island to Brooklyn Station, which is supported.	
	The inclusion of residential density at this site, which is situated within walking distance of a new local centre and established bus service route stops, and cycling distance of an established township with a railway station, provides an opportunity supporting growth and at the same time reducing car dependency in the Hawkesbury River catchment. If parents drop children off to primary school in Brooklyn Hawkesbury River Station, this is not Disability Discrimination Act (DDA) compliant	
	Recommendation From a transport perspective I support the amended PP.	

## TfNSW Issues included in their letter received on 18<sup>th</sup> May 2018

TfNSW Requirements	Response
An assessment of the traffic and safety implications at the Mooney Mooney Interchange, including a review of impacts commercial development to the west of the M1 may have on Interchange operations. It is recommended that a detailed traffic assessment be undertaken, which includes (but not limited to) a microsimulation analysis of the operation of the on and off ramps. Further consultation with Roads and Maritime is recommended to seek advice regarding more detailed scope of work and model specification.	HCM analysis has been undertaken to assess the performance of all motorway segments including on and off- ramps of the M1 - refer <b>section 5.3.3 and Appendix H</b>
The 5 studies noted for completion at Section 6 Key Findings and Recommendations of the Traffic and Transport Review should be included in the updated assessment.	Strategies have been included in <b>sections 6,7,8, and 9</b>
Consideration of the traffic impacts on the existing intersections and the capacity of the road network to safely and efficiently cater for the additional vehicular traffic generated.	The impact on intersections and motoways segments has been assessed for the existing situation and the future 2030 scenario with the development traffic – refer section 5 and appendices G and H
Identify the necessary road network infrastructure upgrades the area requires to maintain existing levels of service on both the local and classified road network. Any proposed changes to the road network will need to be discussed with Council and Roads & Maritime and be supported by a Road Safety Evaluation.	No upgrades are required as a result of the additional trips. Refer <b>section 5 and appendices G and H</b> .
Any other impacts upon the road network including consideration of pedestrian, cyclist and public transport facilities including commuter car parking at local railway stations.	These issues have been discussed in the report – refer <b>section 3,4,5,6, and 7</b> .
Identify feasible options to modify transport impacts if required.	there are no anticipated impacts on the transport services. Capacity of roads, trains, and buses in the area is sufficient to accommodate the additional resulting trips associated with the development.

# **D. 592 Bus Service Timetable**



### How to use this timetable

This timetable provides a snapshot of service information in 24-hour time (e.g. 5am = 05:00, 5pm = 17:00). Information contained in this timetable is subject to change without notice. Please note that timetables do not include minor stops, additional trips for special events, short term changes, holiday timetable changes, real-time information or any disruption alerts.

For the most up-to-date times, use the Trip Planner or Departures at **transportnsw.info** 

### **Real-time planning**

You can plan your trip with real-time information using the Trip Planner or Departures at **transportnsw.info** or by downloading travel apps on your smartphone or tablet.

The Trip Planner, Departures and travel apps offer various features:

- favourite your regular trips
- see where your service is on the route
- get estimated pick-up and arrival times
- receive service updates
- find nearby stations, stops, wharves and routes
- check accessibility information.

Find the latest apps at transportnsw.info/apps

### Accessible services

All new buses are wheelchair-accessible with low-level floors and space for wheelchairs, prams or strollers. Look for the symbol in this timetable. Some older buses may not have all the features you need. There will be more accessible services as older buses are replaced.

### Who is providing my bus services?

The bus services shown in this timetable are run by Transdev NSW.

### Fares

In Sydney and surrounding regions, fares are based on:

- the distance you travel from tap on to tap off
- the mode of transport you choose
- whether you're eligible for a concession fare or free travel
- any Opal benefits such as discounts and capped fares that apply.

You can use an Opal card or a contactless payment to pay for your travel.

### **Opal cards**

An Opal card is a smartcard you keep and reuse. Add value before you travel, and tap on and tap off to pay your fares throughout Sydney, the Blue Mountains, the Central Coast, the Hunter and the Illawarra.

### Which Opal card is right for you?

Adult – Customers 16 years or older who are not entitled to any concessions and normally pay full fare.

**Child/Youth** – For customers aged 4-15 (inclusive), or customers 16 years or older who hold a NSW/ACT Senior Secondary Student Concession Card.

**Gold Senior/Pensioner** – For eligible NSW and interstate seniors, pensioners, war widows/ers and asylum seekers.

**Concession** – For eligible tertiary students, job seekers, apprentices and trainees.

### How to get an Opal card

You can get an Adult or Child/Youth Opal card over the counter at Opal retailers that display the Opal sign **O**. To find your nearest retailer visit **transportnsw.info/opal**.

If you are eligible to travel with concession fares, you can apply for a Gold Senior/Pensioner or Concession Opal card online. Visit **transportnsw.info/opal** for more information.

### **Contactless payments**

If you have an American Express, Mastercard, Visa card or linked device, you can use it to pay for all public transport on the Opal network. Just make sure to tap on and tap off at Opal readers at the beginning and end of your trip.

Always separate your cards when you tap on and tap off so your preferred card is charged.

You will receive the same travel benefits of an Adult Opal card when you tap on and tap off consistently with the same credit card, debit card or linked device. This includes daily, weekly and weekend travel caps, and a \$2 transfer discount when you change between metro/train, ferry, bus and light rail services within 60 minutes. Adult Opal fare pricing applies.

### Find out more at **transportnsw.info/contactless**

### Explanation of definitions and symbols

Wheelchair Accessible





### Valid from: 12 July 2021

#### Creation date: 06 Aug 2021

NOTE: Information is correct on date of download.

Hornsby Station14:05Asquith Station14:08Mount Colah Station14:11Mount Kuring-gai Station14:15Pacific Hwy after Collingridge Way, Berowra14:18Berowra RSL, Pacific Hwy, Berowra14:21Cowan Station14:25	Monday to Friday	
Mount Colah Station14:11Mount Kuring-gai Station14:15Pacific Hwy after Collingridge Way, Berowra14:18Berowra RSL, Pacific Hwy, Berowra14:21	Hornsby Station	14:05
Mount Kuring-gai Station14:15Pacific Hwy after Collingridge Way, Berowra14:18Berowra RSL, Pacific Hwy, Berowra14:21	Asquith Station	14:08
Pacific Hwy after Collingridge Way, Berowra14:18Berowra RSL, Pacific Hwy, Berowra14:21	Mount Colah Station	14:11
Berowra RSL, Pacific Hwy, Berowra 14:21	Mount Kuring-gai Station	14:15
Berowra RSL, Pacific Hwy, Berowra 14:21	Pacific Hwy after Collingridge Way, Berowra	14:18
Cowan Station 14.25	Berowra RSL, Pacific Hwy, Berowra	14:21
	Cowan Station	14:25
Hawkesbury River Station 14:40	Hawkesbury River Station	14:40

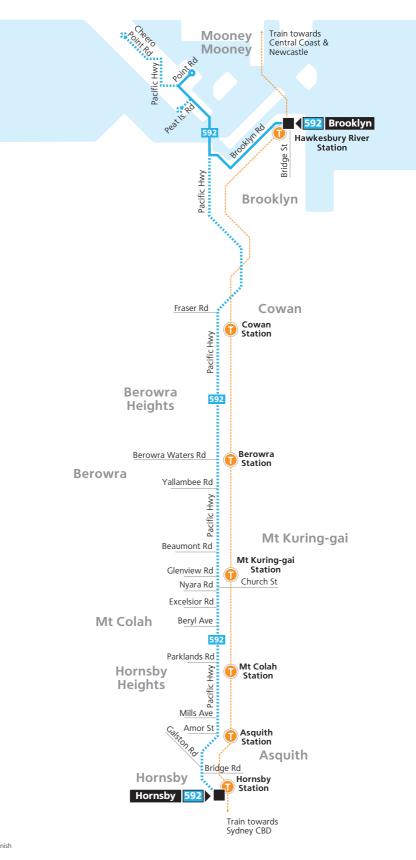
592



Monday to Friday									
Hawkesbury River Station	-	06:50	07:30	07:55	-	09:05	14:49	15:15	16:00
Brooklyn Rd opp Brooklyn Cemetery, Brooklyn	-		07:35		-	09:11	14:54	15:23	16:04
Kangaroo Point Park, Brooklyn	-				-			15:25	
Peat Island Centre, Old Pacific Hwy, Mooney	-	07:00			-	_			
Mooney									
Cheero Point Rd opp Cararma Pkwy, Cheero Point	-	07:10			08:25	09:20		15:36	16:10
Point Rd at Mara Cres, Mooney Mooney		07:15	07:40	08:07	08:30		14:59	15:39	16:15
Peat Island Centre, Old Pacific Hwy, Mooney		_		-				15:45	
Mooney	100000								
Information Bay, Brooklyn Rd, Brooklyn	06:29	07:19	07:44	-	08:35		15:05	15:49	16:19
Hawkesbury River Station	06:34	07:25	07:50	-	08:41		15:14	15:55	16:25
Cowan Station	-	-	-	-	-	09:37	-	-	-
Berowra Station	-	-	-	-	-	09:41	-	-	-
Pacific Hwy opp Foster Way, Berowra	-	-	-	-	-	09:43	-	-	-
Mount Kuring-gai Station	-	-	-	-	-	09:46	-	-	-
Mount Colah Station	-	-	-	-	-	09:50	-	-	-
Asquith Station, Pacific Hwy, Asquith	-	-	-	-	-	09:53	-	-	-
Hornsby Station	-	-	-	-	-	09:57	-	-	-
Monday to Friday									
Hawkesbury River Station	16.42	17:12	17:40	18:08					
Brooklyn Rd opp Brooklyn Cemetery, Brooklyn	16:46	17:21	17:48	18:16					
Cheero Point Rd opp Cararma Pkwy, Cheero Point									
Point Rd at Mara Cres, Mooney Mooney		17:29	17:55	18:23					
Information Bay, Brooklyn Rd, Brooklyn	17:06		17:59	-					
Hawkesbury River Station	17:12	17:40	18:05	-					
	F	Ł	F						
Saturday Hornsby Station	6	-	15:14						
Berowra RSL, Pacific Hwy, Berowra	_	-	15:14						
Pacific Hwy opp Berowra Station, Berowra	08:59	_	15:29						
Cowan Station	08.59	_	15:29						
Hawkesbury River Station	09.05	09:47	15:48						
Brooklyn Rd opp Brooklyn Cemetery, Brooklyn			15:56						
Point Rd at Mara Cres, Mooney Mooney	09:20	10:02	16:03						
Information Bay, Brooklyn Rd, Brooklyn	09:20	10.02	16:03						
Hawkesbury River Station	09:30		16:13						
Cowan Station	- 09.50		16:28						
Berowra Station	_	10:14							
Pacific Hwy opp Foster Way, Berowra	-	10:18	-						
Mount Kuring-gai Station	_	10:20	_						
Mount Colah Station	_	10:23	_						
Asquith Station, Pacific Hwy, Asquith	-	10:27	-						
Hornsby Station	_	10:30	-						
		10.54							

## Route 592





Legend

Bus route Limited service 592 Bus route number Bus route start/finish



Diagrammatic Map Not to Scale

transportnsw.info

# E. Crash Data Analysis (Jan 2013 – Dec 2017)



NOTES: 8317 - Pacific Highway, between Kangaroo Point and Point Road, Mooney Mooney Crash Data - 1 Jan 2013 to 31 Dec 2017

Crash No.	Data Source	Date	Day of Week	Tigitw e	Distance	een Kangaloo Point an Ee Ee E E E E C	Loc Type		Alignment	Weather	Surface Condition	Speed Limit No. of Tus	Tu Type/Obj	Age/Sex	Street Travelling	Speed Travelling Manoeuvre		Degree of Crash-Detailed	illed	Seriously Inj. Moderatelv Ini	_	Uncateg'd Inj.	Factors
Hunter		jion Coast LG/	Δ																				
		ey Moone	-																				
		ific Hwy																					
106286	4 P 2	28/09/2014	Sun	08:00	35 km	S GOSFORD TN	2WY		CRV	Fine	Dry	60 1	M/C	M26	N in PACIFIC HWY	70 Proceedir	g in lane	SC	0	1 (	0 0	0	S
E57221146	6						RUM	83	Off	rt/rt bnd=>	∙obj		S/Bar	rier - G	uardrail								
108821	6 S ´	18/12/2015	Fri	16:00	;	at HEWCASTLE EXP	TJN		STR	Fine	Dry	60 2	4WD	ΜU	S in PACIFIC HWY	Unk Proceedir	g in lane	NC	0	0 (	0 0	0	
E59733445	5						RUM			ar end			4WD	M35	S in PACIFIC HWY	0 Stationary							
111015	7 P - ′	11/06/2016	Sat	10:30		at KOWAN ST	TJN		STR	Fine	Dry	60 2	UTE	M26	N in PACIFIC HWY	30 Turning ri	ght	SC	0	2	1 0	0	
E61529626	6						RUM	21	Rig	ht through			M/C	M54	S in PACIFIC HWY	50 Proceedir	g in lane						
114897	6 P 2	20/07/2017	Thu	14:15		at NUMBER 22 HN	2WY		STR	Fine	Dry	60 3	TRK	M19	W in PACIFIC HWY	10 Forward f	om drive	SC	0	2 (	0 0	0	
E65149257	7						RUM	47	Em	erging fror	n drive		M/C		S in PACIFIC HWY	60 Proceedir	0						
													_M/C		S in PACIFIC HWY	60 Proceedir	<u> </u>						
		29/07/2015	Wed	22:40	10 m	N PEATS FERRY BDGE	2WY		STR	Fine	Dry	80 2	CAR		S in PACIFIC HWY	Unk Pulling ou		NC	0	0 (	0 C	0	
E59500539							RUM			ving parki			_M/C		S in PACIFIC HWY	60 Proceedir							
		01/08/2015	Sat	10:20	;	at POINT RD	TJN		CRV	Fine	Dry	60 2			N in PACIFIC HWY	5 Turning ri	,	MC	0	0	1 0	0	
E57673830							RUM			ht through			M/C		S in PACIFIC HWY	60 Proceedir	g in lane						
		25/12/2014	Thu	12:00	150 m	S POINT RD	2WY		STR	Fine	Dry	60 5	CAR	-	N in PACIFIC HWY	Unk Forward f		NC	0	0 (	0 0	0	
E24362119	94						RUM	39	Oth	er same d	irection				N in PACIFIC HWY	0 Stationary							
													WAG		N in PACIFIC HWY	Unk Forward f							
													WAG		N in PACIFIC HWY	0 Stationary							
													CAR	W25	N in PACIFIC HWY	Unk Forward f	om arive						
Report 7	Fotal	s: Crashes	: 7	Fatal	Crashes(F	FC): 0 Serious Injury Cra	shes(SC):3	3	Moder	ate Injury	Crashes(M	C): 1	Min	or/Othe	r Injury Crashes(OC): 0	Uncategorised Injury C	rashes(UC):	0 No	on-Ca	sualty	Crash	es(NC	;): 3
				Killed	i(K): 0	Seriously Injured	S): 5		Moder	ately Injure	ed(M): 2		Min	or/Othe	r Injured(O): 0	Uncategorised Injured	U): 0						

Crashid dataset 8317 - Pacific Highway, between Kangaroo Point and Point Road, Mooney Mooney Crash Data - 1 Jan 2013 to 31 Dec 2017

Note: Data for the 9 month period prior to the generated date of this report are incomplete and are subject to change.

Crash self reporting, including self reported injuries began Oct 2014. Trends from 2014 are expected to vary from previous yrs. More unknowns are expected in self reported data. Reporting yrs 1996-2004 and 2018 onwards contain uncategorised inj crashes.

### Summary Crash Report



# Croch Turn	•					Crock May	omort			Срасия		7	CASUA		7
# Crash Typ		74 40	Contributi	ng Facto		Crash Move		•	0.007	CRASHE	3	-	CASUA	-	
Car Crash	5	71.4%	Speeding	1	14.3%	Intersection, adjacent approac	nes	0	0.0%	Fatal			Killed	0	0.0%
Light Truck Crash	1	14.3%	Fatigue	0	0.0%	Head-on (not overtaking)		0	0.0%	Serious inj.			Seriously inj.	5	71.4%
Rigid Truck Crash	0	0.0%				Opposing vehicles; turning		2	28.6%	Moderate inj.			Moderately inj.	2	28.6%
Articulated Truck Crash	0	0.0%				U-turn		0	0.0%	Minor/Other inj.			Minor/Other inj.	0	0.0%
'Heavy Truck Crash	(0)	(0.0%)	Weat			Rear-end		1	14.3%	Uncategorised inj.			Uncategorised in		0.0%
Bus Crash	0	0.0%	Fine	7	100.0%	Lane change		0	0.0%	Non-casualty		3 42.9%	^ Unrestrained	0	0.0%
"Heavy Vehicle Crash	(0)	(0.0%)	Rain	0	0.0%	Parallel lanes; turning		0	0.0%	Self Reported Crash		2 28.57%	<ul> <li>A Belt fitted but not w fitted to position OR</li> </ul>		
Emergency Vehicle Crash	0	0.0%	Overcast	0	0.0%	Vehicle leaving driveway		1	14.3%						
Motorcycle Crash	5	71.4%	Fog or mist	0	0.0%	Overtaking; same direction		0	0.0%	Time Group	%	of Day	Crashes		alties
Pedal Cycle Crash	0	0.0%	Other	0	0.0%	Hit parked vehicle		0	0.0%	00:01 - 02:59		0.0% 12.5%	1	2017	2
Pedestrian Crash	0	0.0%	Road Surfac	e Condit	ion	Hit railway train		0	0.0%	03:00 - 04:59		0.0% 12.5% 0.0% 8.3%	1	2016	3
'Rigid or Artic. Truck " Heavy Tr			Wet	0	0.0%	Hit pedestrian		0	0.0%	05:00 - 05:59		0.0% 0.3% 0.0% 4.2%	3	2015	1
# These categories are NOT me		clusive		Ũ		Permanent obstruction on road	b	0	0.0%	06:00 - 06:59		0.0% 4.2% 0.0% 4.2%	2	2014	1
Location Ty	ре		Dry	7	100.0%	Hit animal		0	0.0%	07:00 - 07:59		0.0% 4.2% 0.0% 4.2%			
*Intersection	3	42.9%	Snow or ice	0	0.0%	Off road, on straight		0	0.0%	08:00 - 08:59		4.3% 4.2%			
Non intersection	4	57.1%	Natural I	iahtina		Off road on straight, hit object		0	0.0%	09:00 - 09:59		4.3% 4.2% 0.0% 4.2%			
* Up to 10 metres from an inters	section					Out of control on straight		0	0.0%	10:00 - 10:59		3.0% 4.2% 8.6% 4.2%			
[			Dawn	0	0.0%	Off road, on curve		0	0.0%	11:00 - 11:59		5.0% 4.2% 0.0% 4.2%			
Collision Ty	/pe		Daylight	6	85.7%	Off road on curve, hit object		1	14.3%	12:00 - 12:59		4.2% 4.3% 4.2%			
Single Vehicle	1	14.3%	Dusk	0	0.0%	Out of control on curve		0	0.0%	13:00 - 13:59		4.3% 4.2% 0.0% 4.2%			
Multi Vehicle	6	85.7%	Darkness	1	14.3%	Other crash type		2	28.6%	14:00 - 14:59		4.3% 4.2%	McLean Periods	s % W	/eek
						Speed Limit				15:00 - 15:59		4.2% 0.0% 4.2%	A (	0.0%	17.9%
Road Classific			40 km/h or less	0	0.0	•	1	14.3%		16:00 - 16:59		4.3% 4.2%	В	14.3%	7.1%
Freeway/Motorway	0	0.0%	50 km/h zone	0	0.0	% 90 km/h zone	0	0.0%		17:00 - 17:59		4.2% 0.0% 4.2%	C 2	2 28.6%	17.9%
State Highway		100.0%	60 km/h zone	6	85.79	% 100 km/h zone	0	0.0%		18:00 - 18:59		0.0% 4.2% 0.0% 4.2%	D	2 28.6%	3.5%
Other Classified Road	0	0.0%	70 km/h zone	0			0	0.0%		18:00 - 18:59		0.0% 4.2% 0.0% 4.2%	E (	0.0%	3.6%
Unclassified Road	0	0.0%					-			20:00 - 19:59		0.0% 4.2% 0.0% 8.3%	F (	0.0%	10.7%
~ 07:30-09:30 or 14:30-17:00	) on scho	ol days	~ 40km/h or less	0	0.0%	~ School Travel Time Involvem	ent	1	14.3%			J.0% 8.3% 4.3% 8.3%	G	14.3%	7.1%
			Day of t	he Week						22:00 - 24:00	1 14	+.3% 0.3%	H (	0.0%	7.1%
Monday 0 0.0%	S Wedne	esdav	1 14.3% Friday		1 14.3	% Sunday 1 14.3% W	EEKEN	<b>ND</b> 3	42.9%	Street Lighting Off/Ni	I %	of Dark	1 '	14.3%	12.5%
-	Thurs	-	2 28.6% Saturda	ay		% WEEKDAY 4 57.1%		-			in Dar	k 100.0%	J	0.0%	10.7%
-		•		<u> </u>											
New Year 0	0.0% E	actor	0 0.0	#⊦ Quee≀ %	loliday Po	1 14.3% Christmas		1 14.3%	Easter S	<b>H</b> 0 0.0%	Sont	/Oct. SH	1 14.3%		
		aster		% Queer		0 0.0% January SH			June/Ju			mber SH	1 14.3%		
πασι. μαγ 0	0.070 A		0.0		. Day			0.070	June/Ju	<b>iy Gii</b> 0.0%	Dece		1 14.370		

Crashid dataset 8317 - Pacific Highway, between Kangaroo Point and Point Road, Mooney Mooney Crash Data - 1 Jan 2013 to 31 Dec 2017

Note: Data for the 9 month period prior to the generated date of this report are incomplete and are subject to change.

Crash self reporting, including self reported injuries began Oct 2014. Trends from 2014 are expected to vary from previous yrs. More unknowns are expected in self reported data. Reporting yrs 1996-2004 and 2018 onwards contain uncategorised inj crashes.

Percentages are percentages of all crashes. Unknown values for each category are not shown on this report.



NOTES: 8317 - Pacific Motorway (including off/on ramps), between Kangaroo Point and the southbound safety ramp, Mooney Mooney Crash Data - 1 Jan 2013 to 31 Dec 2017

31 Dec 2017												<b>T</b>
Crash No. Data Source Date Day of Week	Time Distance	ID Feature	Loc Type	Alignment	Weather	Surface Condition	Speed Limit No. of Tus	Tu Type/Obj	Age/Sex	Street Travelling	Speed Travelling Manoeuvre	Degree of Crash-Detailed Killed Seriously Inj. Moderately Inj. Minor/Other Inj Uncateg'd Inj. Factors
Hunter Region Central Coast LGA Mooney Mooney Newcastle Exp												31
1001549 P 27/09/2013 Fri	11:27	at HAWKESBURY RIVER BDG	E DF	STR	Fine	Drv	110 2	CAR	F24	S in NEWCASTLE EXP	20 Proceeding in lane	MC 0 0 2 0 0
E51884720				-	ear end	,				S in NEWCASTLE EXP	20 Proceeding in lane	
Newcastle Exp				00 10								
840661 P 20/06/2013 Thu	06:05	10 m N HAWKESBURY RIV BDGE	D F	STR	Fine	Drv	110 2	CAR	F26	S in NEWCASTLE EXP	50 Proceeding in lane	NC 0 0 0 0 0
E51824376				-	ear end	,				S in NEWCASTLE EXP	0 Stationary	
1042096 P 10/09/2014 Wed	15:10 1	00 m N HAWKESBURY RIVER BDG		STR	Fine	Drv	110 6			N in NEWCASTLE EXP	110 Veering right	NC 0 0 0 0 0
E56388669				-	ear end	,				N in NEWCASTLE EXP	110 Proceeding in lane	
20000000				00 10				CAR		N in NEWCASTLE EXP	110 Proceeding in lane	
								CAR	M52	N in NEWCASTLE EXP	110 Proceeding in lane	
								WAG	M33	N in NEWCASTLE EXP	110 Proceeding in lane	
								4WD	F28	N in NEWCASTLE EXP	110 Proceeding in lane	
1000940 P 15/11/2013 Fri	21:00 2	00 m N HAWKESBURY RIVER BDG	E DF	STR	Overcast	Wet	110 1	CAR	M30	S in NEWCASTLE EXP	100 Proceeding in lane	NC 0 0 0 0 0 F
E53200150			RUM	71 Off	f rd left => o	obj		S/Barr	ier - Gu	ardrail		
843807 P 01/07/2013 Mon	10:55	at HAWKESBURY RIVER BDG	E DF	STR	Fine	Dry	110 2	TRK	M79	N in NEWCASTLE EXP	30 Proceeding in lane	NC 0 0 0 0 0
E51920218			RUM	30 Re	ear end			WAG	M60	N in NEWCASTLE EXP	15 Proceeding in lane	
845320 P 05/07/2013 Fri	17:35	at HAWKESBURY RIVER BDG	E DF	STR	Fine	Dry	110 8	CAR	F23	N in NEWCASTLE EXP	100 Proceeding in lane	MC 0 0 2 0 0 S
E52516869			RUM	30 Re	ear end			WAG	F37	N in NEWCASTLE EXP	0 Stationary	
								CAR	F64	N in NEWCASTLE EXP	100 Proceeding in lane	
								CAR		N in NEWCASTLE EXP	80 Proceeding in lane	
								CAR		N in NEWCASTLE EXP	0 Stationary	
								UTE		N in NEWCASTLE EXP	100 Proceeding in lane	
								TRK		N in NEWCASTLE EXP	90 Proceeding in lane	
								CAR	F22	N in NEWCASTLE EXP	95 Proceeding in lane	



Crash No. Data Source		Day of Week	Time	Distance	ID Feature	Loc Type	Alignment	Weather	Surface Condition	Speed Limit No. of Tus	Tu Type/Obj	Age/Sex	Street Travelling	Speed Travelling	Manoeuvre	Degree of Crash-Detailed	Killed Soriouchy Ini	seriousiy inj. Moderately inj.	Minor/Other Inj.	Uncateg'd Inj.	S A
1020658 P E105889602	17/04/2014	Thu	18:25		at HAWKESBURY RIVER BDG	GE DF RUM	-	Fine Rear end	Dry	110 10	OMV CAR TRK CAR CAR 4WD 4WD CAR	F U F27 M59 F U F35 F42 M67 F29	N in NEWCASTLE EXP N in NEWCASTLE EXP	0 S 105 P 105 P 0 S 100 P 100 P 100 P 100 P	roceeding in lane tationary roceeding in lane roceeding in lane tationary roceeding in lane roceeding in lane roceeding in lane	00	0	0 0	4	0	
E55037054	12/06/2014				at HAWKESBURY RIVER BDG	RUM		lear end	Dry	110 3	TRK CAR CAR TRK	M54 F21 M44 M44	N in NEWCASTLE EXP N in NEWCASTLE EXP N in NEWCASTLE EXP N in NEWCASTLE EXP	80 P 0 S	roceeding in lane roceeding in lane tationary tationary	NC	0	0 0	0	0	
1032234 P E55015926	12/06/2014	Thu	18:10		at HAWKESBURY RIVER BDC	SE DF RUM	STR 30 R	Fine tear end	Dry	110 7	WAG CAR TRK CAR TRK WAG UTE	F59 M40 F22 M32	N in NEWCASTLE EXP N in NEWCASTLE EXP	0 S 100 P 0 S 0 S 0 S	roceeding in lane tationary roceeding in lane tationary tationary tationary tationary	OC	0	0 0	1	0	
1029806 P E55646063	12/06/2014	Thu	18:14		at HAWKESBURY RIVER BDG	GE DF RUM:		Fine Rear end	Dry	110 7	CAR 4WD CAR CAR TRK 4WD CAR	M31 M39	N in NEWCASTLE EXP N in NEWCASTLE EXP	80 P 80 P 80 P 80 P 80 P	roceeding in lane roceeding in lane roceeding in lane roceeding in lane roceeding in lane roceeding in lane roceeding in lane	NC	0	0 0	0	0	
1048413 P E55719306	03/10/2014	Fri	06:00		at HAWKESBURY RIVER BDC	GE DF RUM		Fine lear end	Dry	110 8	TRK CAR TRK WAG CAR TRK 4WD CAR	M34 M69 M27 M42 M42 M31 M57 F41	S in NEWCASTLE EXP S in NEWCASTLE EXP	0 S 100 P 100 P 100 P 100 S 0 S 0 S	roceeding in lane tationary roceeding in lane roceeding in lane roceeding in lane tationary tationary roceeding in lane	MC	0	0 1	1	0	



	Date Source Date	Day of Week	Time	Distance	ID Feature	Loc Type	Alignment	Weather	Surface Condition	Speed Limit No. of Tus	Tu Type/Obj	Age/Sex	Street Travelling	Speed Travelling Manoeuvre	Degree of Crash-Detailed	Killed Seriolisly Ini	Moderately Ini	Minor/Other Inj.	Uncateg'd Inj.	Factors
																				01
1059585 \$	S 26/12/2014	Fri	12:18		at HAWKESBURY RIVER BDG	E DF	STI	R Fine	Dry	110 4	CAR	M27	N in NEWCASTLE EXP	Unk Proceeding in lane	MC	0	0	1 0	0 (	
E57202679						RUM	30	Rear end			4WD	M41	N in NEWCASTLE EXP	Unk Proceeding in lane						
											CAR	F27	N in NEWCASTLE EXP	Unk Proceeding in lane						
											ATKR	M50	N in NEWCASTLE EXP	Unk Proceeding in lane						
1061995 F	P 01/01/2015	Thu	11:30		at HAWKESBURY RIVER BDG	E DF	STI	R Fine	Dry	110 2	CAR	F51	N in NEWCASTLE EXP	100 Proceeding in lane	MC	0	0	1 1	0	
E56390117						RUM	30	Rear end			CAR	F30	N in NEWCASTLE EXP	100 Proceeding in lane						
1074663 F	P 04/07/2015	Sat	15:30		at HAWKESBURY RIVER BDG	E DF	STI	R Fine	Dry	110 2	CAR	F29	N in NEWCASTLE EXP	110 Proceeding in lane	MC	0	0	1 0	0 0	
E289107693						RUM	33	Lane sideswipe	•		LOR	M71	N in NEWCASTLE EXP	100 Proceeding in lane						
1075992 F	P 14/07/2015	Tue	10:25		at HAWKESBURY RIVER BDG	E DF	STI	R Fine	Dry	110 3	WAG	M75	N in NEWCASTLE EXP	90 Proceeding in lane	SC	0	1 (	0 0	0	
E58418750						RUM	30	Rear end			CAR	M64	N in NEWCASTLE EXP	0 Stationary						
											TRK	M42	N in NEWCASTLE EXP	90 Proceeding in lane						
1099642 \$	S 23/04/2016	Sat	13:45		at HAWKESBURY RIVER BDG	E DF	STI	R Fine	Dry	110 4	CAR	F66	N in NEWCASTLE EXP	Unk Proceeding in lane	NC	0	0 0	0 0	0	
E63060285						RUM	30	Rear end			CAR	M64	N in NEWCASTLE EXP	0 Stationary						
											4WD	M27	N in NEWCASTLE EXP	0 Stationary						
											CAR	F59	N in NEWCASTLE EXP	Unk Proceeding in lane						
1105292 \$	S 11/06/2016	Sat	10:10		at HAWKESBURY RIVER BDG	E DF	STI	R Fine	Dry	110 3	TRK	M66	S in NEWCASTLE EXP	Unk Proceeding in lane	OC	0	0 0	0 1	0	
E61856873						RUM	62	Accident			CAR	F46	S in NEWCASTLE EXP	0 Broken down						
											CAR	M81	S in NEWCASTLE EXP	0 Stationary						
1111081 F	P 11/06/2016	Sat	10:15		at HAWKESBURY RIVER BDG	E DF	STI	R Fine	Dry	110 3	WAG	M49	S in NEWCASTLE EXP	110 Proceeding in lane	SC	0	1 (	0 1	0	
E61411307						RUM	62	Accident			TRK	M66	S in NEWCASTLE EXP	0 Broken down						
											PED		N in NEWCASTLE EXP	Against traffic, not edge						
1109903 F	P 06/07/2016	Wed	17:50		at HAWKESBURY RIVER BDG	E DF	STI	R Fine	Dry	110 3	CAR	M47	N in NEWCASTLE EXP	90 Proceeding in lane	MC	0	0	1 0	) 0	
E308446993						RUM	30	Rear end			TRK	M55	N in NEWCASTLE EXP	70 Proceeding in lane						
											LOR	M54	N in NEWCASTLE EXP	80 Proceeding in lane						
1165687 F	P 23/12/2017	Sat	16:50		at HAWKESBURY RIVER BDG	E DF	STI	R Fine	Dry	110 3	TRK	F18	N in NEWCASTLE EXP	Unk Proceeding in lane	MC	0	0	1 0	0 0	
E67183508						RUM	62	Accident			SEM	M32	N in NEWCASTLE EXP	0 Broken down						
											VAN	M71	N in NEWCASTLE EXP	110 Proceeding in lane						
1072377 \$	S 23/05/2015	Sat	14:30	50 m	N HAWKESBURY RIVER BDG	E DF	STI	R Fine	Dry	110 2	TRK	M63	N in NEWCASTLE EXP	Unk Proceeding in lane	MC	0	0	1 0	0 0	
E58312254						RUM	30	Rear end			VAN	ΜU	N in NEWCASTLE EXP	Unk Proceeding in lane						
1084638 F	P 29/09/2015	Tue	16:30	80 m	N HAWKESBURY RIVER BDG	E DF	STI	R Fine	Dry	110 3	UTE	F35	S in NEWCASTLE EXP	100 Veering right	MC	0	0	1 0	0 0	
E59311338						RUM	34	Lane change ri	ght		CAR	M68	S in NEWCASTLE EXP	100 Proceeding in lane						
								5	-		CAR	M24	S in NEWCASTLE EXP	100 Proceeding in lane						
1019592 F	P 28/03/2014	Fri	18:15	100 m	N HAWKESBURY RIVER BDG	E DF	STI	R Overcast	Dry	110 2	CAR	M33	N in NEWCASTLE EXP	110 Proceeding in lane	NC	0	0 0	0 0	0	
E54793079						RUM	30	Rear end			CAR	F24	N in NEWCASTLE EXP	Unk Proceeding in lane						
														-						



Crash No. Data Source Date Day of Week Time	Distance ID Feature	Loc Type	Weather	Surface Condition	Speed Limit No. of Tus	Tu Type/Obj	Age/Sex	Street Travelling	Speed Travelling Manoeuvre	Degree of Crash-Detailed Killed Seriously Inj. Moderately Inj. Uncateg'd Inj. Factors
										SF
1099236 P 01/04/2016 Fri 06:30	100 m N HAWKESBURY RIVER BE	GE DF S	STR Fine	Dry	110 3	OMV	UU	S in NEWCASTLE EXP	Unk Proceeding in lane	NC 0 0 0 0 0
E60232015		RUM: 30	Rear end			TRK	M23	S in NEWCASTLE EXP	Unk Proceeding in lane	
						TRK	M38	S in NEWCASTLE EXP	20 Proceeding in lane	
1013771 P 17/02/2014 Mon 06:24	400 m N HAWKESBURY RIVER BE	GE DF S	STR Raining	Wet	110 4	TRK	M21	S in NEWCASTLE EXP	Unk Proceeding in lane	OC 0 0 0 2 0
E53886212		RUM: 30	Rear end				F31	S in NEWCASTLE EXP	0 Stationary	
							M56	S in NEWCASTLE EXP	80 Proceeding in lane	
							M70	S in NEWCASTLE EXP	85 Veering left	
1002073 P 01/12/2013 Sun 18:00	1 km N HAWKESBURY RIVER BE		CRV Fine	Dry			M25	N in NEWCASTLE EXP	100 Veering right	NC 0 0 0 0 0 S
E54072741		RUM: 88	Out of cont on	bend			F75	N in NEWCASTLE EXP	100 Proceeding in lane	
		~					F19	N in NEWCASTLE EXP	100 Proceeding in lane	
1031056 P 05/02/2014 Wed 20:30	1 km N HAWKESBURY RIVER BE		STR Fine	Dry			M38	S in NEWCASTLE EXP	100 Incorrect side	SC 0 4 1 0 0 S F
E53931721		RUM: 20	Head on				F57	N in NEWCASTLE EXP	Unk Proceeding in lane	
1049186 P 23/10/2014 Thu 13:25	1 km N HAWKESBURY RIVER BE		CRV Fine	Dry	90 1			S in NEWCASTLE EXP	85 Proceeding in lane	SC 0 1 0 0 0
E56764969		RUM: 81	Off left/rt bnd=			Emban				
1071109 P 01/05/2015 Fri 17:45	1 km N HAWKESBURY RIVER BE		STR Raining	Wet	100 3			N in NEWCASTLE EXP	50 Proceeding in lane	NC 0 0 0 0 0
E58270449		RUM: 30	Rear end				UU	N in NEWCASTLE EXP	Unk Proceeding in lane	
		~	<u></u>				M34	N in NEWCASTLE EXP	40 Proceeding in lane	
1088583 P 24/11/2015 Tue 03:20	1 km N HAWKESBURY RIVER BE		CRV Raining	Wet	110 2			N in NEWCASTLE EXP	110 Proceeding in lane	SC 0 1 0 0 0 S F
E60010262		RUM: 81	Off left/rt bnd=			SBDA		N in NEWCASTLE EXP	0 Parked other	
1017363 P 22/02/2014 Sat 10:30	300 m S HAWKSBURY RIVER AME	-	STR Fine	Dry	110 2			N in NEWCASTLE EXP	70 Proceeding in lane	NC 0 0 0 0 0
E55957085			Rear end					N in NEWCASTLE EXP	0 Stationary	
1019948 P 28/03/2014 Fri 18:50	2 km S JOLLS BDGE		CRV Raining	Wet	110 3			N in NEWCASTLE EXP	110 Proceeding in lane	OC 0 0 0 1 0
E54169433		RUM: 30	Rear end				M52		0 Stationary	
1116324 P 17/08/2016 Wed 06:20	400 m N MOONEY MOONEY REST	A DF S	STR Fine	Drv	110 3		M24 M30	N in NEWCASTLE EXP	0 Stationary	SC 0 1 0 2 0
	400 M N MOONET MOONET REST			Dry			M26	S IN NEWCASTLE EXP	Unk Proceeding in lane 0 Stationary	SC 0 1 0 2 0
E61735306		RUM: 30	Rear end					S in NEWCASTLE EXP	0 Stationary 0 Stationary	
830846 P 23/03/2013 Sat 10:15	at NTH END HAWKES BDGE	DF S	STR Fine	Dry	110 2			N in NEWCASTLE EXP	60 Veering right	MC 0 0 1 0 0
E51138246		RUM: 34	Lane change r			4WD		N in NEWCASTLE EXP	105 Proceeding in lane	
830847 P 23/03/2013 Sat 10:15	at NTH END HAWKES BDGE		STR Fine	Drv	110 2			N in NEWCASTLE EXP	100 Proceeding in lane	NC 0 0 0 0 0
E51138246		RUM: 30	Rear end	Diy				N in NEWCASTLE EXP	70 Proceeding in lane	
841858 P 19/06/2013 Wed 06:00	290 m S NTH END HAWKES BDGE		STR Overcast	Wet	110 2		M47	S in NEWCASTLE EXP	60 Proceeding in lane	OC 0 0 0 1 0
E51885640		RUM: 30	Rear end	**51				S in NEWCASTLE EXP	60 Proceeding in lane	
1128722 S 24/02/2017 Fri 13:30	400 m N OLD PACIFIC HIGHWAY		CRV Fine	Dry	90 2			N in NEWCASTLE EXP	Unk Proceeding in lane	NC 0 0 0 0 0
E63211425		RUM: 33	Lane sideswip	,			F24	N in NEWCASTLE EXP	Unk Proceeding in lane	
LUJZ   1420 				5			1 27			



Crash No.	source	of Week		nce	Feature	Type	Alignment	her	Surface Condition	d Limit f Tus		<b>šex</b>	Street Travelling	Speed Travelling	anoeuvre	Degree of Crash-Detailed	_	Seriously Inj.	Moderately Inj.		Jncateg'd Inj.	SI
rasł	Date	Day c	Time	Distance	ů Č	oc T	lign	Neather	urfa ond	Speed	lu Ty	ge/S	tree rave	peer	lano	egre rash	Killed	erio	lode	linor	nca	Factor
		Δ	<u> </u>	<u> </u>	<b>\</b>	Ľ.	<	5	ပပ	ທ Z	Ē	◄	ο F	SΗ	Σ	ΔU	X	Ś	Σ	Σ	<u> </u>	SF
	P 31/05/2017	Wed	10:27	300 m	S OLD PACIFIC HIGHWAY O	P DF	CR		Dry	110 3			S in NEWCASTLE EXP		Proceeding in lane	MC	0	0	1	0	0	
E65490515						RUM	30	Rear end			WAG		S in NEWCASTLE EXP		Proceeding in lane							
							=				OMV		S in NEWCASTLE EXP		Proceeding in lane							
	O3/04/2013	Wed	17:50	45 m	N PACIFIC HIGHWA OP	DF	CR	5	Wet	90 -	1 UTE		N in NEWCASTLE EXP	95 H	Proceeding in lane	NC	0	0	0	0	0	S
E50588515						RUM	85	Off rt/lft bnd=>					to 2014)									
829176 F	P 01/03/2013	Fri	13:00	300 m	N PACIFIC HIGHWA OP	DF	CR	5	Wet	110	1 CAR	M21	N in NEWCASTLE EXP	95 F	Proceeding in lane	NC	0	0	0	0	0	S
E50773376						RUM	85	Off rt/lft bnd=>	obj		Fence	e (prior	to 2014)									
832556 F	P 04/04/2013	Thu	10:25	10 m	S PACIFIC HIGHWA OP	DF	CR	V Raining	Wet	110	1 CAR	M30	S in NEWCASTLE EXP	110 F	Proceeding in lane	NC	0	0	0	0	0	S
E51913739						RUM	83	Off rt/rt bnd=>	obj		Fence	e (prior	to 2014)									
856820	P 02/11/2013	Sat	11:20	75 m	S PACIFIC HIGHWA OP	DF	CR	V Fine	Dry	110	1 TRK	M37	N in NEWCASTLE EXP	110 F	Proceeding in lane	NC	0	0	0	0	0	F
E53445179						RUM	85	Off rt/lft bnd=>	obj		Fence	e (prior	to 2014)									
846918	P 22/07/2013	Mon	06:20	25 m	N PACIFIC HIGHWA TO	DF	ST	R Fine	Dry	110 9	9 TRK	M55	S in NEWCASTLE EXP	110 F	Proceeding in lane	OC	0	0	0	5	0	
E52549968						RUM	30	Rear end			CAR	M25	S in NEWCASTLE EXP	110 F	Proceeding in lane							
											TRK	M28	S in NEWCASTLE EXP	110 F	Proceeding in lane							
											TRK	M35	S in NEWCASTLE EXP	110 F	Proceeding in lane							
											CAR	M35	S in NEWCASTLE EXP	110 F	Proceeding in lane							
											TRK	M49	S in NEWCASTLE EXP	Unk F	Proceeding in lane							
											WAG	M36	S in NEWCASTLE EXP	100 F	Proceeding in lane							
											UTE	M39	S in NEWCASTLE EXP	110 F	Proceeding in lane							
											UTE	M40	S in NEWCASTLE EXP	110 F	Proceeding in lane							
846910 F	22/07/2013	Mon	06:30	35 m	N PACIFIC HIGHWA TO	DF	ST	R Fine	Dry	110 2	2 CAR	M45	S in NEWCASTLE EXP	Unk F	Proceeding in lane	NC	0	0	0	0	0	
E52549968						RUM	30	Rear end			4WD	M58	S in NEWCASTLE EXP	0 5	Stationary							
846917	P 22/07/2013	Mon	06:30	45 m	N PACIFIC HIGHWA TO	DF	ST	R Fine	Dry	110	2 TRK	M57	S in NEWCASTLE EXP	110 F	Proceeding in lane	NC	0	0	0	0	0	
E52549968						RUM	30	Rear end			TRK	M47	S in NEWCASTLE EXP	Unk F	Proceeding in lane							
1011055 F	P 17/02/2014	Mon	06:24		at PACIFIC HIGHWAY TO	DF	ST	R Overcast	Wet	110 3	3 OMV	UU	S in NEWCASTLE EXP	Unk F	Proceeding in lane	NC	0	0	0	0	0	
E53675752						RUM	30	Rear end			CAR	M50	S in NEWCASTLE EXP	0 5	Stationary							
											CAR	M41	S in NEWCASTLE EXP	0 5	Stationary							l
1084988	S 17/11/2015	Tue	07:00	100 m	N PACIFIC HIGHWAY OP	DF	ST	R Fine	Dry	110 2	2 CAR	M52	S in NEWCASTLE EXP	Unk F	Proceeding in lane	NC	0	0	0	0	0	
E258573894						RUM	30	Rear end			CAR	M44	S in NEWCASTLE EXP	Unk F	Proceeding in lane							
1026999	P 20/04/2014	Sun	18:30	200 m	N PACIFIC HIGHWAY OP	DF	ST	R Fine	Dry	110 2	2 CAR	FU	S in NEWCASTLE EXP	Unk F	Proceeding in lane	MC	0	0	1	0	0	
E55161862						RUM	30	Rear end			CAR	M35	S in NEWCASTLE EXP	0 \$	Stationary							

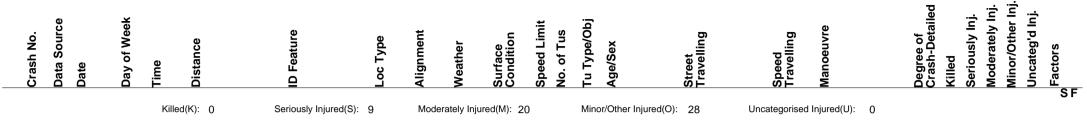


Crash No.	Data Source Date	Day of Week	Time	Distance	ID Feature	Loc Type	Alignment	Weather	Surface Condition	Speed Limit No. of Tus	Tu Type/Obj	Age/Sex	Street Travelling	Speed Travelling	Manoeuvre	Degree of Crash-Detailed Killed	Seriously Ini.	Moderately Inj.	Minor/Other Inj.	Uncateg'd Inj.	Factors 4
	S 26/05/2015	Tue	17:40	500 m N	PACIFIC HIGHWAY OP	DF	CRV	Fine	Dry	110 5	CAR	M22	N in NEWCASTLE EXP		Proceeding in lane	NC (	0 0	) 0	0	0	
E58145004						RUM	30 R	ear end			CAR	F35	N in NEWCASTLE EXP		Proceeding in lane						
											CAR	F26	N in NEWCASTLE EXP		Proceeding in lane						
											CAR	FU			Proceeding in lane						
1021204	P 27/04/2014	Sup	12:00		PACIFIC HIGHWAY OP	D F	CRV	Raining	Wet	90 1	CAR CAR	_M U 	N in NEWCASTLE EXP		Proceeding in lane	MC	0 0	) 1	0	0	
	P 27/04/2014	Sun	13:00	630 m N	PACIFIC RIGRIVAT OP			0		90 1				90 F	roceeding in lane	IVIC (	5 0	, 1	0	0	5
E54373036								ff rt/rt bnd=					ardrail								
	S 06/04/2015	Mon	11:00	10 m S	PACIFIC HIGHWAY OP	DF	CRV	Fine	Dry	110 5	CAR		S in NEWCASTLE EXP		Other forward	00 0	0 0	) ()	1	0	
E57662557						RUM	39 C	ther same	direction		CAR	F25			Stationary						
											CAR 4WD	F38 M51	S in NEWCASTLE EXP S in NEWCASTLE EXP		Other forward Other forward						
											400 4WD	M20	S in NEWCASTLE EXP		Other forward						
1056177 9	S 23/01/2015	 Fri	14.30	100 m S	PACIFIC HIGHWAY OP	D F	CRV	Fine	Dry	110 3		 F23	N in NEWCASTLE EXP		Proceeding in lane	NC (	<u> </u>	) 0		0	
E56665227	0 20/01/2010		14.00					ear end	Diy	110 0	4WD	M33	N in NEWCASTLE EXP		Proceeding in lane		5 0	, 0	0	0	
E30003227						RUW	30 F	ear enu			TRK	M31	N in NEWCASTLE EXP		Proceeding in lane						
1094134 9	S 25/02/2016	Thu	17:15	150 m S	PACIFIC HIGHWAY OP	D F	CRV	Fine	Dry	110 2	4WD	M24	N in NEWCASTLE EXP		Proceeding in lane	NC (	<u> </u>	0 0	0	0	
E63153088	0 20/02/2010			100111 0				ear end	2.)		4WD	M59	N in NEWCASTLE EXP		Proceeding in lane				Ũ	Ũ	
	S 29/09/2015	Tue	16:45	200 m S	PACIFIC HIGHWAY OP	DF	CRV	Fine	Drv	110 2		M67	S in NEWCASTLE EXP		Proceeding in lane	NC (	0 0	0 (	0	0	
E59672979	0 20/00/2010	100	10.10	200111 0				ear end	Diy	110 2	CAR	MU	S in NEWCASTLE EXP		Proceeding in lane				Ũ	Ũ	
	S 04/09/2016	Sun	16:02	500 m S	PACIFIC HIGHWAY OP	D F	STR	Fine	Dry	110 3		F49	S in NEWCASTLE EXP		Proceeding in lane	OC (	) (	0	1	0	
E63062065	0 0 1/00/2010	Curr	10.02	000111 0				ear end	Diy	110 0	TRK	M53	S in NEWCASTLE EXP		Proceeding in lane	00	5 0	, 0	•	Ŭ	
L03002003						NOW	30	earenu			4WD	M38	S in NEWCASTLE EXP		Proceeding in lane						
1152178 \$	S 03/08/2017	Thu	06:15	600 m S	PACIFIC HIGHWAY OP	D F	STR	Fine	Drv	110 2		M56	S in NEWCASTLE EXP		Proceeding in lane	OC	0 0	0 0		0	
E65517944						RUM	-	ear end	,		CAR	M50	S in NEWCASTLE EXP		Stationary						
	P 17/02/2014	Mon	06.24	at	PACIFIC HIGHWAY TO	DF		Overcas	t Wet	110 3		M26	S in NEWCASTLE EXP		Proceeding in lane	NC (	<u> </u>	0 0		0	
E53883345			00.21					ear end			CAR	M24	S in NEWCASTLE EXP		Stationary				Ũ	Ũ	
20000040							55 F				CAR	M52	S in NEWCASTLE EXP		Stationary						
1119591 \$	S 13/11/2016	Sun	10:25	at	PACIFIC HIGHWAY TO	D F	STR	Fine	Dry	110 2		M43	S in NEWCASTLE EXP		Proceeding in lane	NC (	0 0	0 0	0	0	
E62926748					-			ther same	,		SEM	M37	S in NEWCASTLE EXP		Other forward						
	S 27/06/2017	Tue	17:50	at	PACIFIC HIGHWAY TO	DF	STR	Fine	Dry	110 2	TRK	M30	N in NEWCASTLE EXP		Proceeding in lane	OC(	0 0	0 (	1	0	
E64489427							-	ear end	,		CAR	F65	N in NEWCASTLE EXP		Proceeding in lane			U		-	
	S 27/02/2015	Fri	16:50	50 m N	PACIFIC HIGHWAY TO	DF	STR	Fine	Dry	110 4	4WD		N in NEWCASTLE EXP		Proceeding in lane	OC (	<u> </u>	) 0	1	0	
E56946352								ear end	2.,		CAR	UU	N in NEWCASTLE EXP		Proceeding in lane			5		-	
2000-0002							00 1				4WD	M54	N in NEWCASTLE EXP		Proceeding in lane						
											CAR		N in NEWCASTLE EXP		Proceeding in lane						



Crash No. Data Source		Day of Week	Time	Distance	ID Feature	Loc Type	Alignment	Weather	Surface Condition	Speed Limit No. of Tus	Tu Type/Obj	Age/Sex	Street Travelling	Speed Travelling Manoeuvre	Degree of Crash-Detailed	Killed	Seriously Inj. Moderately Ini	Minor/Other Inj.	Uncateg'd Inj.	Factors
																				SF
1059545 S 23/	02/2015	Mon	06:05	300 m N	N PACIFIC HIGHWAY TO	D F	CR	V Raining	Wet	110 3	TRK	M43	S in NEWCASTLE EXP	Unk Proceeding in lane	NC	0	0	0 0	0 0	
E56288320						RUM	30	Rear end			UTE	M48	S in NEWCASTLE EXP	Unk Proceeding in lane						
											WAG	M36	S in NEWCASTLE EXP	Unk Proceeding in lane						
1046963 P 25/	08/2014	Mon	13:30	460 m N	PACIFIC HIGHWAY TO	DF	STR	R Fine	Dry	90 1	BDBI	M27	S in NEWCASTLE EXP	90 Proceeding in lane	NC	0	0	0 0	0 0	
E56477139						RUM	71	Off rd left =>	obj		Emba	nkment								
1066233 P 25/	04/2015	Sat	21:15	670 m N	N PACIFIC HIGHWAY TO	DF	CR	V Fine	Dry	110 1	CAR	F44	N in NEWCASTLE EXP	Unk Proceeding in lane	NC	0	0	0 0	0 0	S
E57636413						RUM	83	Off rt/rt bnd=:	>obj		S/Bar	rier - G	Jardrail							
1123238 S 19/	12/2016	Mon	06:15	10 m S	PACIFIC HIGHWAY TO	DF	STR	R Overcast	Wet	110 2	CAR	F25	S in NEWCASTLE EXP	Unk Proceeding in lane	NC	0	0	0 0	0 0	
E63420961						RUM	30	Rear end			CAR	M29	S in NEWCASTLE EXP	Unk Proceeding in lane						
1065798 S 18/	04/2015	Sat	20:30	100 m S	PACIFIC HIGHWAY TO	DF	STR	R Fine	Dry	110 1	CAR	M45	S in NEWCASTLE EXP	Unk Proceeding in lane	MC	0	0	1 0	0 0	
E57460736						RUM	73	Off rd rght =>	• obj		S/Bar	rier - Co	oncr/Jersey							
1018235 P 20/	03/2014	Thu	06:15	200 m S	PACIFIC HIGHWAY TO	D F	STR	R Fine	 Dry	110 4	TRK	M44	S in NEWCASTLE EXP	90 Proceeding in lane	OC	0	0	0 2	2 0	
E54444238						RUM	30	Rear end	,		WAG	M28	S in NEWCASTLE EXP	0 Stationary						
						-					OMV	UU	S in NEWCASTLE EXP	0 Stationary						
											CAR	UU	S in NEWCASTLE EXP	Unk Proceeding in lane						
1146272 S 24/	08/2017	Thu	05:50	250 m S	PACIFIC HIGHWAY TO	DF	CR	V Fine	Dry	110 3	CAR	M48	S in NEWCASTLE EXP	Unk Proceeding in lane	NC	0	0	0 0	0 0	/
E65195703						RUM	30	Rear end			CAR	M52	S in NEWCASTLE EXP	0 Stationary						
											CAR	M28	S in NEWCASTLE EXP	0 Stationary						
1129979 S 15/	02/2017	Wed	05:30	300 m S	PACIFIC HIGHWAY TO	DF	STR	R Fine	Dry	110 2	TRK	UU	S in NEWCASTLE EXP	Unk Proceeding in lane	OC	0	0	0 1	1 0	
E64032966						RUM	30	Rear end			CAR	M47	S in NEWCASTLE EXP	0 Stationary						
1023452 P 21/	04/2014	Mon	11:30	390 m S	S PACIFIC HIGHWAY TO	DF	STR	R Fine	Dry	110 4	CAR	M39	S in NEWCASTLE EXP	80 Proceeding in lane	NC	0	0	0 0	0 0	
E55387165						RUM	30	Rear end			TRK	M29	S in NEWCASTLE EXP	Unk Proceeding in lane						
											CAR	M30		100 Proceeding in lane						
											CAR		S in NEWCASTLE EXP	0 Stationary						
1028571 P 07/	06/2014	Sat	13:00	100 m N	I PEATS FERRY ROAD OP	DF	STR		Dry	110 2		F27	N in NEWCASTLE EXP	90 Proceeding in lane	MC	0	0	1 0	0 0	
E54711232						RUM		Rear end			CAR	M28	N in NEWCASTLE EXP	Unk Proceeding in lane						
1028797 P 07/	06/2014	Sat	13:00	500 m N	PEATS FERRY ROAD OP	DF	CR	V Fine	Dry	60 2	SEM	M36		100 Proceeding in lane	NC	0	0	0 0	0 0	S
E55298358						RUM		Lane sideswi			CAR	F47	N in NEWCASTLE EXP	80 Proceeding in lane						
1052480 P 01/	01/2015	Thu	11:38	300 m S	<b>5 PRINCES HIGHWAY TO</b>	DF	STE	R Fine	Dry	110 3		M85	S in NEWCASTLE EXP	110 Proceeding in lane	MC	0	0	1 0	0 0	
E56852148						RUM	30	Rear end			CAR		S in NEWCASTLE EXP	Unk Proceeding in lane						
								·				M26	S in NEWCASTLE EXP	0 Stationary						
1017089 P 17/	02/2014	Mon	06:25	50 m N	N REST AREA TO	DF	CR	0	Wet	110 2		M44	S in NEWCASTLE EXP	70 Proceeding in lane	NC	0	0	0 0	0 0	S
E54813108						RUM	30	Rear end			CAR	F69	S in NEWCASTLE EXP	0 Stationary						
Report Totals:	Crashes:	73	Fatal	Crashes(FC	C): 0 Serious Injury Crash	hes(SC):6	6 Ma	derate Injury	Crashes(N	/IC): 17	Mine	or/Othe	r Injury Crashes(OC): 14	Uncategorised Injury Crashes(UC	;): 0 No	on-Ca	sualty	Cras	hes(N(	C): 36





Crashid dataset 8317 - Pacific Motorway (including off/on ramps), between Kangaroo Point and the southbound safety ramp, Mooney Mooney Crash Data - 1 Jan 2013 to 31 Dec 2017 **Note:** Data for the 9 month period prior to the generated date of this report are incomplete and are subject to change.

Crash self reporting, including self reported injuries began Oct 2014. Trends from 2014 are expected to vary from previous yrs. More unknowns are expected in self reported data. Reporting yrs 1996-2004 and 2018 onwards contain uncategorised inj crashes.

### Summary Crash Report



# Crash Type			<b>—</b>	Crash Mo	vement			CRASHES		73	CASUA		57
	65 89.0		ng Factors			0	0.0%	Fatal	0		Killed	0	57 0.0%
Car Crash		opecung	11 15.19		cnes	0			•			-	
Light Truck Crash	31 42.5		4 5.5%			T O		Serious inj.	6		Seriously inj.	9	15.8%
Rigid Truck Crash	4 5.5° 6 8.2°			Opposing vehicles; turning		0	0.0%	Moderate inj. Minor/Other ini.	17 14		Moderately inj.	20	35.1%
Articulated Truck Crash	• •		thor			•	0.0%		14	19.2%	Minor/Other inj.	. 28	49.1%
'Heavy Truck Crash	(10) (13.75			Rear-end		50	68.5%	Uncategorised inj.	•	0.0%	Uncategorised in	<b>j.</b> 0	0.0%
Bus Crash	0 0.0		57 78.19			5	6.8%	Non-casualty	36	49.3%	^ Unrestrained	1	1.8%
"Heavy Vehicle Crash	(10) (13.79	, I	10 13.79			0	0.0%	Self Reported Crash	21	28.77%	^ Belt fitted but not w fitted to position OR	No helmet w	orn
Emergency Vehicle Crash	1 1.4		6 8.29			0	0.0%				Crashes	Casu	
Motorcycle Crash	0 0.0		0 0.09	Overtaking; same direction		0	0.0%	Time Group	% of	Day			
Pedal Cycle Crash	0 0.0		0 0.00			0	0.0%	00:01 - 02:59	0.0	% 12.5%	7	2017	5
Pedestrian Crash	1 1.4	Road Surfa	ce Condition	Hit railway train		0	0.0%	03:00 - 04:59		% 8.3%	10	2016	8
<ul><li>Rigid or Artic. Truck " Heavy Tru # These categories are NOT mut</li></ul>			15 20.5%	Hit pedestrian		0	0.0%	05:00 - 05:59	2 2.7	% 4.2%	17	2015	11
<b>u</b>		Dry	58 79.5%	Permanent obstruction on roa	ad	0	0.0%	<b>06:00 - 06:59</b> 10	5 21.9	% 4.2%	23	2014	22 11
Location Typ		Crimer in in a	0 0.0%	Hit animai		0	0.0%	07:00 - 07:59	1 1.4	% 4.2%	16	2013	11
*Intersection	0 0.0	/0	0.07			0	0.0%	08:00 - 08:59	0.0	% 4.2%			
Non intersection	73 100.0	<sup>70</sup> Natural	Lighting	Off road on straight, hit object	t	3	4.1%	09:00 - 09:59	0.0	% 4.2%			
* Up to 10 metres from an interse	ection	Dawn	12 16.4%	Out of control on straight		0	0.0%	<b>10:00 - 10:59</b> 10	) 13.7	% 4.2%			
Collision Typ	20	Daylight	44 60.39	Off road, on curve		0	0.0%	11:00 - 11:59	6 8.2	% 4.2%			
			44 00.37	Off road on curve, hit object		8	11.0%	12:00 - 12:59	1.4	% 4.2%			
Single Vehicle				Out of control on curve		1	1.4%	13:00 - 13:59	3 11.0	% 4.2%	McLean Periods	0/ 14	/eek
Multi Vehicle	63 86.3	% Darkness	13 17.89	Other crash type		5	6.8%	14:00 - 14:59	2 2.7	% 4.2%			
Road Classifica	ation			Speed Limit				15:00 - 15:59	2 2.7	% 4.2%	A 20		17.9%
Freeway/Motorway	73 100.0	40 km/h or less		0% 80 km/h zone		0.0%		16:00 - 16:59	5 6.8	% 4.2%		0.0%	7.1%
State Highway	0 0.0	50 km/h zone	0 0	0% 90 km/h zone		6.8%		17:00 - 17:59	7 9.6	% 4.2%	C 15		17.9%
Other Classified Road	0 0.0	60 km/h zone	1 1	4% <b>100 km/h zone</b>	2	2.7%		18:00 - 18:59	3 11.0	% 4.2%	D 10		3.5%
Unclassified Road	0 0.0	70 km/h zone	0 0	0% <b>110 km/h zone</b>	65 8	9.0%		19:00 - 19:59	0.0	% 4.2%	E		3.6%
								20:00 - 21:59	4 5.5	% 8.3%	F 8		10.7%
~ 07:30-09:30 or 14:30-17:00 (	on school days		0 0.0%	~ School Travel Time Involver	nent	2	2.7%	22:00 - 24:00		% 8.3%	G 10		7.1%
		-	the Week								H O		7.1%
•	Wednesday	8 11.0% Friday			NEEKEND	19	26.0%	Street Lighting Off/Nil	% of	-		0.070	12.5%
<b>Tuesday</b> 7 9.6%	Thursday	13 17.8% Sature	l <b>ay</b> 14 19	.2% <b>WEEKDAY</b> 54 74.0%				4 of 13 ir	n Dark	30.8%	J	2 2.7%	10.7%
			#Holiday	Periods									
New Year 2 2	2.7% Easter	4 5.5	W Queen's BD	4 5.5% Christmas	2	2.7% I	Easter S	H 7 9.6% S	Sept./Oc	ct. SH	4 5.5%		
Aust. Day 1 1	.4% Anzac	<b>Day</b> 3 4.1	% Labour Day	1 1.4% January SH	3	4.1%	June/Jul	ySH 4 5.5% I	Decemb	er SH	2 2.7%		

Crashid dataset 8317 - Pacific Motorway (including off/on ramps), between Kangaroo Point and the southbound safety ramp, Mooney Mooney Crash Data - 1 Jan 2013 to 31 Dec 2017

Note: Data for the 9 month period prior to the generated date of this report are incomplete and are subject to change.

Percentages are percentages of all crashes. Unknown values for each category are not shown on this report.

Crash self reporting, including self reported injuries began Oct 2014. Trends from 2014 are expected to vary from previous yrs. More unknowns are expected in self reported data. Reporting yrs 1996-2004 and 2018 onwards contain uncategorised inj crashes.



NOTES: 8317 - Pacific Motorway (including off/on ramps), between Kangaroo Point and the southbound safety ramp, Mooney Mooney Crash Data - 1 Jan 2013 to 31 Dec 2017

Crash No.	Data Source	Date	Day of Week	Time	Distance	ID Feature	Loc Type	Alignment	Weather	Surface Condition	Speed Limit No. of Tus	Tu Type/Obj	Age/Sex	Street Travelling	Speed Travelling Manoeuvre	Degree of Crash-Detailed Killed Seriously Inj. Moderately Inj. Minor/Other Inj. Uncateg'd Inj. Factors
	tral Co looney	on bast LG / Moon castle E	ey													
115872 E6664692		7/12/201	7 Thu	17:30	8	at PEATS FERRY RD	RDB RUM: 3	STR 0 R	Fine ear end	Dry	60 3	4WD CAR TRK	M U F64 M34	N in NEWCASTLE EXP N in NEWCASTLE EXP W in PEATS FERRY RD	Unk Proceeding in lane 0 Stationary Unk Turning right	NC 0 0 0 0 0
113376 E3225583		4/04/201	7 Fri	14:40	80 m 💲	S PEATS FERRY RD	OTH RUM 3	STR 0 R	Fine ear end	Dry	60 3	CAR TRK CAR	M39	N in NEWCASTLE EXP N in NEWCASTLE EXP N in NEWCASTLE EXP	Unk Proceeding in lane 0 Stationary 0 Stationary	NC 0 0 0 0 0
Report <sup>-</sup>	Totals	: Crashe	es: 2		Crashes(F (K): 0	C): 0 Serious Injury Cra Seriously Injured			erate Injury erately Injur		IC): 0			r Injury Crashes(OC): 0 r Injured(O): 0	Uncategorised Injury Crashes(UC): Uncategorised Injured(U): 0	0 Non-Casualty Crashes(NC): 2

Crashid dataset 8317 - Northbound Pacific Motorway off/on ramp, up to intersection with Pacific Highway, Mooney Mooney Crash Data - 1 Jan 2013 to 31 Dec 2017

Note: Data for the 9 month period prior to the generated date of this report are incomplete and are subject to change.

Crash self reporting, including self reported injuries began Oct 2014. Trends from 2014 are expected to vary from previous yrs. More unknowns are expected in self reported data. Reporting yrs 1996-2004 and 2018 onwards contain uncategorised inj crashes.

### Summary Crash Report



									]					
# Crash Type		Co	ontributing Facto	ors	Crash Move	ement			CRASHE	S	2	CASUAL	TIES	0
Car Crash	2 100	0% Speeding	0	0.0%	Intersection, adjacent approach	nes	0	0.0%	Fatal	C	0.0%	Killed	0	0.0%
Light Truck Crash	2 100	0% Fatigue	0	0.0%	Head-on (not overtaking)		0	0.0%	Serious inj.	0	0.0%	Seriously inj.	0	0.0%
Rigid Truck Crash	0 0.	0%			Opposing vehicles; turning		0	0.0%	Moderate inj.	0	0.0%	Moderately inj.	0	0.0%
Articulated Truck Crash	0 0.	0%			U-turn		0	0.0%	Minor/Other inj.	0	0.0%	Minor/Other inj.	0	0.0%
'Heavy Truck Crash	(0) (0.	0%)	Weather		Rear-end		2	100.0%	Uncategorised inj.	C	0.0%	Uncategorised inj	0	0.0%
Bus Crash	0 0.	0% Fine	2	100.0%	Lane change		0	0.0%	Non-casualty	2	2 100.0%	^ Unrestrained	0	0.0%
"Heavy Vehicle Crash	(0) (0.	0%) Rain	0	0.0%	Parallel lanes; turning		0	0.0%	Self Reported Crash		2 100%	^ Belt fitted but not wo fitted to position OR N		
Emergency Vehicle Crash	0 0.	0% Overcast	0	0.0%	Vehicle leaving driveway		0	0.0%						
Motorcycle Crash	0 0.	0% Fog or mi	<b>st</b> 0	0.0%	Overtaking; same direction		0	0.0%	Time Group	%	of Day	Crashes	Casu	alties
Pedal Cycle Crash	0 0.	0% Other	0	0.0%	Hit parked vehicle		0	0.0%	00:01 - 02:59		0% 12.5%	2	2017	0
Pedestrian Crash		0% Roa	d Surface Condi	tion	Hit railway train		0	0.0%	03:00 - 04:59		0% 12.5% 0% 8.3%			
' Rigid or Artic. Truck " Heavy Truc		Bus	0	0.0%	Hit pedestrian		0	0.0%	05:00 - 05:59		0% 0.3% 0% 4.2%			
# These categories are NOT mut		Dry	2	100.0%	Permanent obstruction on road	l	0	0.0%	06:00 - 06:59		0% 4.2%			
Location Type				0.0%	Hit animal		0	0.0%	07:00 - 07:59		0% 4.2%			
*Intersection		.0 /0		0.076	Off road, on straight		0	0.0%	08:00 - 08:59		0% 4.2%			
Non intersection	1 50	.0%	Natural Lighting		Off road on straight, hit object		0	0.0%	09:00 - 09:59		0% 4.2%			
* Up to 10 metres from an intersec	ction	Dawn	0	0.0%	Out of control on straight		0	0.0%	10:00 - 10:59		0% 4.2%			
Oslisian Tra	•		0		Off road, on curve		0	0.0%	11:00 - 11:59		0% 4.2%			
Collision Typ		Daylight	1	50.0%	Off road on curve, hit object		0	0.0%	12:00 - 12:59		0% 4.2%			
Single Vehicle		.0% Dusk	1	50.0%	Out of control on curve		0	0.0%	13:00 - 13:59	0 0.	0% 4.2%	Mel een Darie de	0/ 14	laak
Multi Vehicle	2 100	0% Darkness	0	0.0%	Other crash type		0	0.0%	14:00 - 14:59	1 50.	0% 4.2%	McLean Periods		Veek
Road Classifica	tion				Speed Limit				15:00 - 15:59	0 0.	0% 4.2%	<b>A</b> 0	0.0%	17.9%
Freeway/Motorway	2 100.	40 km/h o	rless C	0.0	% 80 km/h zone	0 0	0.0%		16:00 - 16:59	0 0.	0% 4.2%	<b>B</b> 0	0.0%	7.1%
State Highway		0% 50 km/h z	one C				).0%		17:00 - 17:59	1 50.	0% 4.2%	<b>C</b> 1	50.0%	17.9%
Other Classified Road		0% 60 km/h z	one 2				0.0%		18:00 - 18:59	0 0.	0% 4.2%	<b>D</b> 0	0.0%	3.5%
Unclassified Road		0% <b>70 km/h z</b>	one C	0.0	% 110 km/h zone	0 0	).0%		19:00 - 19:59	0 0.	0% 4.2%	<b>E</b> 0	0.0%	3.6%
				0.00/				0.001	20:00 - 21:59	0 0.	0% 8.3%	<b>F</b> 0	0.0%	10.7%
~ 07:30-09:30 or 14:30-17:00 o	n school day	/s ~ 40km/h o	•	0.0%	~ School Travel Time Involveme	ent	0	0.0%	22:00 - 24:00	0 0.	0% 8.3%	<b>G</b> 1	50.0%	7.1%
		_	Day of the Week						Otwood Link the a Office		( Deul	H 0	0.0%	7.1%
-	Wednesda		Friday		% Sunday 0 0.0% WE	EEKEND	0	0.0%	Street Lighting Off/Ni		f Dark	<b>J</b> 0	0.0% 0.0%	12.5%
Tuesday 0 0.0%	Thursday	1 50.0%	Saturday	0 0.0	% <b>WEEKDAY</b> 2*********				0 of 0	in Dark	0.0%	J U	0.0%	10.7%
L			#	Holiday P	eriods									1
<b>New Year</b> 0 0.	.0% Easte	r	1 50.0% Quee		0 0.0% Christmas	0	0.0% I	Easter S	<b>SH</b> 1 50.0%	Sept./C	Oct. SH	0 0.0%		ļ
Aust. Day 0 0.	.0% <b>Anza</b>	: Day	0 0.0% <b>Labo</b>	ur Day	0 0.0% January SH	0	0.0%	June/Ju	ly SH 0 0.0%	Decem	ber SH	0 0.0%		ļ
														I

Crashid dataset 8317 - Northbound Pacific Motorway off/on ramp, up to intersection with Pacific Highway, Mooney Mooney Crash Data - 1 Jan 2013 to 31 Dec 2017

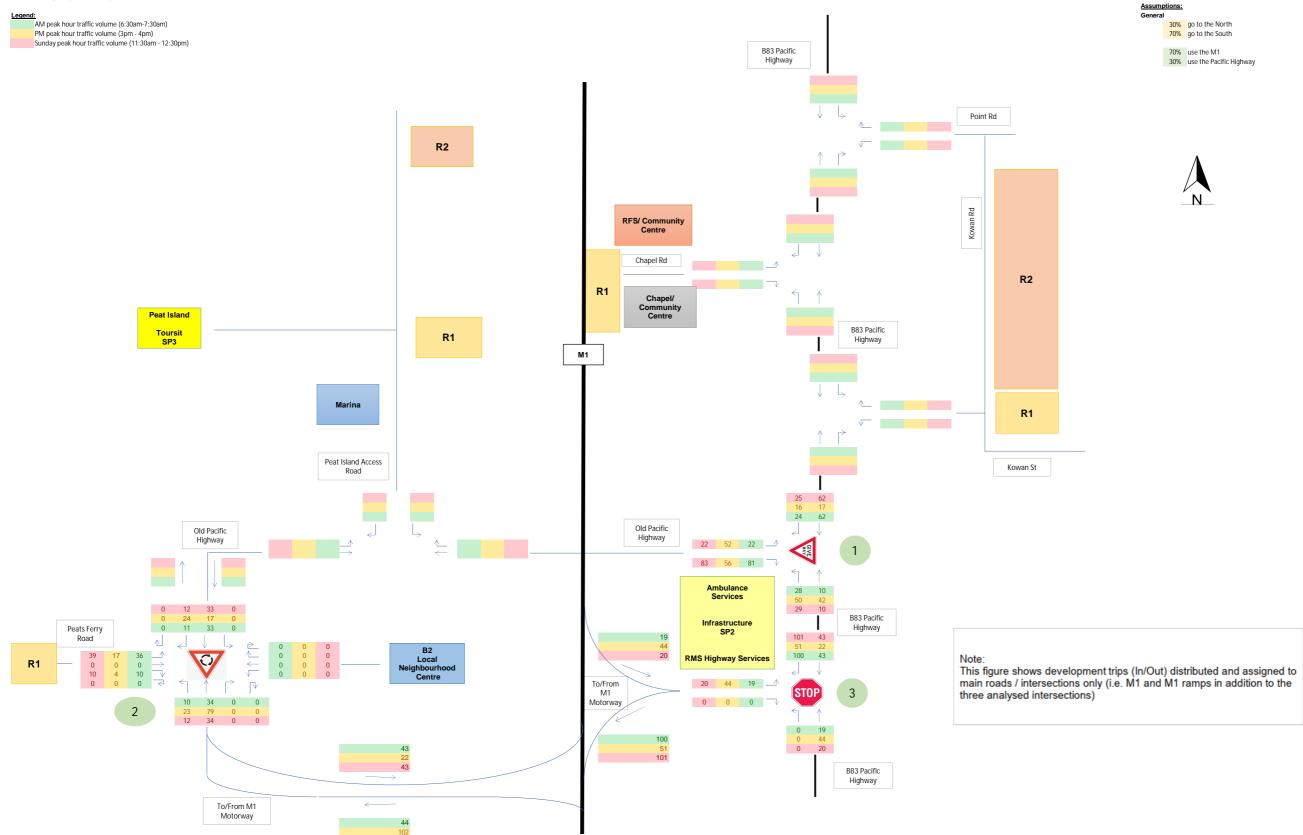
Note: Data for the 9 month period prior to the generated date of this report are incomplete and are subject to change.

Crash self reporting, including self reported injuries began Oct 2014. Trends from 2014 are expected to vary from previous yrs. More unknowns are expected in self reported data. Reporting yrs 1996-2004 and 2018 onwards contain uncategorised inj crashes.

Percentages are percentages of all crashes. Unknown values for each category are not shown on this report.

# F. Trip Assignment

#### Total Trips (In + Out)



#### Assumptions

Gen

era		
	30%	go to the North
	70%	go to the South
	70%	use the M1
	30%	use the Pacific Highway

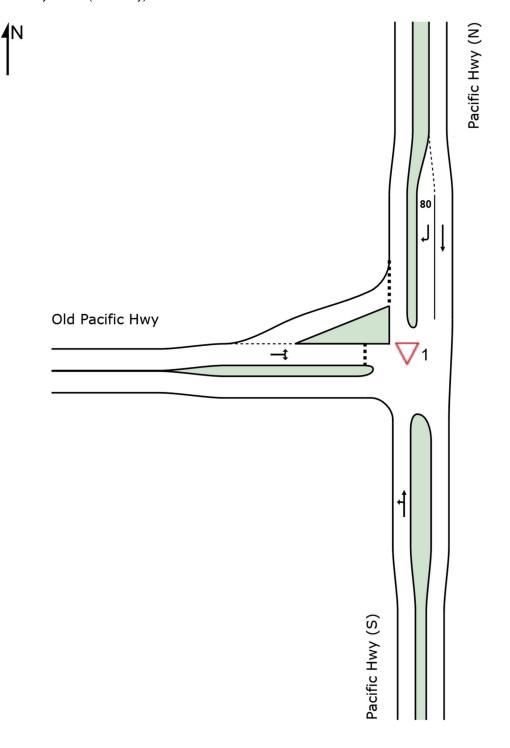


# **G. SIDRA Analysis**

## SITE LAYOUT

## ablaSite: 1 Pacific Hwy / Old Pacific Hwy

New Site Site Category: (None) Giveway / Yield (Two-Way)



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## Site: 1 [AM (Weekday) Pacific Hwy / Old Pacific Hwy - 2018]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ment	Performa	nce - \	Vehicl	es							
Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Turri	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South:	Pacifi	c Hwy (S)										
1	L2	34	10.0	0.029	5.7	LOS A	0.0	0.0	0.00	0.38	0.00	53.6
2	T1	18	10.0	0.029	0.0	LOS A	0.0	0.0	0.00	0.38	0.00	56.6
Approa	ach	52	10.0	0.029	3.7	NA	0.0	0.0	0.00	0.38	0.00	54.7
North:	Pacific	c Hwy (N)										
8	T1	62	5.0	0.033	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	60.0
9	R2	6	16.0	0.004	5.8	LOS A	0.0	0.1	0.14	0.54	0.14	50.5
Approa	ach	68	6.0	0.033	0.5	NA	0.0	0.1	0.01	0.05	0.01	59.2
West:	Old Pa	acific Hwy										
10	L2	13	16.0	0.051	5.8	LOS A	0.2	1.6	0.13	0.56	0.13	51.9
12	R2	42	15.0	0.051	6.3	LOS A	0.2	1.6	0.13	0.56	0.13	50.6
Approa	ach	55	15.2	0.051	6.2	LOS A	0.2	1.6	0.13	0.56	0.13	50.9
All Veh	nicles	175	10.1	0.051	3.2	NA	0.2	1.6	0.04	0.31	0.04	55.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## ▽Site: 1 [PM (Weekday) Pacific Hwy / Old Pacific Hwy - 2018]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ment l	Performa	ince - \	Vehicl	es							
Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Turri	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South:	Pacifi	c Hwy (S)										
1	L2	53	6.0	0.053	5.6	LOS A	0.0	0.0	0.00	0.32	0.00	54.4
2	T1	44	7.0	0.053	0.0	LOS A	0.0	0.0	0.00	0.32	0.00	57.1
Approa	ach	97	6.5	0.053	3.1	NA	0.0	0.0	0.00	0.32	0.00	55.8
North:	Pacific	Hwy (N)										
8	T1	55	10.0	0.030	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	60.0
9	R2	25	4.0	0.016	5.8	LOS A	0.1	0.5	0.20	0.55	0.20	50.9
Approa	ach	80	8.1	0.030	1.8	NA	0.1	0.5	0.06	0.17	0.06	57.2
West:	Old Pa	cific Hwy										
10	L2	27	4.0	0.067	5.8	LOS A	0.3	1.9	0.17	0.56	0.17	52.2
12	R2	49	2.0	0.067	6.4	LOS A	0.3	1.9	0.17	0.56	0.17	51.1
Approa	ach	77	2.7	0.067	6.1	LOS A	0.3	1.9	0.17	0.56	0.17	51.5
All Veh	nicles	254	5.8	0.067	3.6	NA	0.3	1.9	0.07	0.35	0.07	55.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## Site: 1 [Peak Hour (Sunday) Pacific Hwy / Old Pacific Hwy - 2018]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ment	Performa	nce - '	Vehicl	es							
Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Tun	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South:	Pacifi	c Hwy (S)										
1	L2	73	2.0	0.117	5.6	LOS A	0.0	0.0	0.00	0.19	0.00	55.8
2	T1	148	2.0	0.117	0.0	LOS A	0.0	0.0	0.00	0.19	0.00	58.2
Approa	ach	221	2.0	0.117	1.8	NA	0.0	0.0	0.00	0.19	0.00	57.5
North:	Pacific	c Hwy (N)										
8	T1	127	2.0	0.066	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	60.0
9	R2	18	2.0	0.012	6.2	LOS A	0.1	0.4	0.32	0.55	0.32	50.6
Approa	ach	145	2.0	0.066	0.8	NA	0.1	0.4	0.04	0.07	0.04	58.9
West:	Old Pa	acific Hwy										
10	L2	43	2.0	0.216	6.2	LOS A	0.9	6.6	0.41	0.66	0.41	51.2
12	R2	149	2.0	0.216	7.9	LOS A	0.9	6.6	0.41	0.66	0.41	50.0
Approa	ach	193	2.0	0.216	7.5	LOS A	0.9	6.6	0.41	0.66	0.41	50.2
All Veh	nicles	559	2.0	0.216	3.5	NA	0.9	6.6	0.15	0.32	0.15	55.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## Site: 1 [AM (Weekday) Pacific Hwy / Old Pacific Hwy - 2030 without Dev.]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ment l	Performa	nce - \	Vehicl	es							
Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Turri	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South:	Pacifi	c Hwy (S)										
1	L2	38	10.0	0.033	5.7	LOS A	0.0	0.0	0.00	0.38	0.00	53.6
2	T1	20	10.0	0.033	0.0	LOS A	0.0	0.0	0.00	0.38	0.00	56.6
Approa	ach	58	10.0	0.033	3.7	NA	0.0	0.0	0.00	0.38	0.00	54.7
North:	Pacific	Hwy (N)										
8	T1	71	5.0	0.037	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	60.0
9	R2	7	16.0	0.005	5.8	LOS A	0.0	0.2	0.15	0.54	0.15	50.5
Approa	ach	78	6.0	0.037	0.6	NA	0.0	0.2	0.01	0.05	0.01	59.1
West:	Old Pa	cific Hwy										
10	L2	15	16.0	0.059	5.8	LOS A	0.2	1.8	0.13	0.56	0.13	51.8
12	R2	47	15.0	0.059	6.4	LOS A	0.2	1.8	0.13	0.56	0.13	50.5
Approa	ach	62	15.2	0.059	6.3	LOS A	0.2	1.8	0.13	0.56	0.13	50.8
All Veh	nicles	198	10.1	0.059	3.3	NA	0.2	1.8	0.05	0.31	0.05	55.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## Site: 1 [PM (Weekday) Pacific Hwy / Old Pacific Hwy - 2030 without Dev.]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ment	Performa	nce - \	Vehicl	es							
Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Tunn	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South:	Pacifi	c Hwy (S)										
1	L2	60	6.0	0.061	5.6	LOS A	0.0	0.0	0.00	0.32	0.00	54.4
2	T1	51	7.0	0.061	0.0	LOS A	0.0	0.0	0.00	0.32	0.00	57.1
Approa	ach	111	6.5	0.061	3.1	NA	0.0	0.0	0.00	0.32	0.00	55.8
North:	Pacific	Hwy (N)										
8	T1	62	10.0	0.034	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	60.0
9	R2	28	4.0	0.018	5.8	LOS A	0.1	0.6	0.22	0.55	0.22	50.9
Approa	ach	91	8.1	0.034	1.8	NA	0.1	0.6	0.07	0.17	0.07	57.2
West:	Old Pa	cific Hwy										
10	L2	31	4.0	0.077	5.8	LOS A	0.3	2.2	0.19	0.56	0.19	52.2
12	R2	56	2.0	0.077	6.5	LOS A	0.3	2.2	0.19	0.56	0.19	51.0
Approa	ach	86	2.7	0.077	6.3	LOS A	0.3	2.2	0.19	0.56	0.19	51.4
All Veh	nicles	287	5.9	0.077	3.6	NA	0.3	2.2	0.08	0.35	0.08	55.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## ablaSite: 1 [Peak Hour (Sunday) Pacific Hwy / Old Pacific Hwy - 2030 without Dev.]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ment	Performa	nce - '	Vehicl	es							
Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Turn	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South:	Pacifi	c Hwy (S)										
1	L2	82	2.0	0.132	5.6	LOS A	0.0	0.0	0.00	0.19	0.00	55.8
2	T1	168	2.0	0.132	0.0	LOS A	0.0	0.0	0.00	0.19	0.00	58.2
Approa	ach	251	2.0	0.132	1.8	NA	0.0	0.0	0.00	0.19	0.00	57.5
North:	Pacific	c Hwy (N)										
8	T1	144	2.0	0.076	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	60.0
9	R2	20	2.0	0.014	6.3	LOS A	0.1	0.4	0.34	0.56	0.34	50.6
Approa	ach	164	2.0	0.076	0.8	NA	0.1	0.4	0.04	0.07	0.04	58.9
West:	Old Pa	acific Hwy										
10	L2	48	2.0	0.257	6.3	LOS A	1.1	8.0	0.45	0.69	0.45	50.8
12	R2	169	2.0	0.257	8.4	LOS A	1.1	8.0	0.45	0.69	0.45	49.6
Approa	ach	218	2.0	0.257	7.9	LOS A	1.1	8.0	0.45	0.69	0.45	49.9
All Veh	nicles	633	2.0	0.257	3.7	NA	1.1	8.0	0.17	0.33	0.17	55.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## $\nabla$ Site: 1 [AM (Weekday) Pacific Hwy / Old Pacific Hwy - 2030 with Dev.]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ment	Performa	nce - '	Vehicl	es							
Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	TUITI	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South:	Pacifi	c Hwy (S)										
1	L2	67	10.0	0.056	5.7	LOS A	0.0	0.0	0.00	0.40	0.00	53.4
2	T1	31	10.0	0.056	0.0	LOS A	0.0	0.0	0.00	0.40	0.00	56.5
Approa	ach	98	10.0	0.056	3.9	NA	0.0	0.0	0.00	0.40	0.00	54.4
North:	Pacific	Hwy (N)										
8	T1	135	5.0	0.072	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	60.0
9	R2	33	16.0	0.022	6.0	LOS A	0.1	0.8	0.21	0.55	0.21	50.3
Approa	ach	167	7.1	0.072	1.2	NA	0.1	0.8	0.04	0.11	0.04	58.2
West:	Old Pa	cific Hwy										
10	L2	38	16.0	0.186	5.9	LOS A	0.8	6.3	0.21	0.59	0.21	51.0
12	R2	134	15.0	0.186	7.5	LOS A	0.8	6.3	0.21	0.59	0.21	49.8
Approa	ach	172	15.2	0.186	7.1	LOS A	0.8	6.3	0.21	0.59	0.21	50.0
All Veh	nicles	437	11.0	0.186	4.1	NA	0.8	6.3	0.10	0.36	0.10	54.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## ♥ Site: 1 [PM (Weekday) Pacific Hwy / Old Pacific Hwy - 2030 with Dev.]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Movement Performance - Vehicles												
Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Turri	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South: Pacific Hwy (S)												
1	L2	96	6.0	0.108	5.6	LOS A	0.0	0.0	0.00	0.29	0.00	54.7
2	T1	101	7.0	0.108	0.0	LOS A	0.0	0.0	0.00	0.29	0.00	57.4
Approa	ach	197	6.5	0.108	2.7	NA	0.0	0.0	0.00	0.29	0.00	56.2
North:	Pacific	Hwy (N)										
8	T1	45	10.0	0.025	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	60.0
9	R2	80	4.0	0.055	6.2	LOS A	0.2	1.8	0.31	0.57	0.31	50.6
Approa	ach	125	6.2	0.055	3.9	NA	0.2	1.8	0.20	0.37	0.20	54.1
West:	West: Old Pacific Hwy											
10	L2	85	4.0	0.192	6.0	LOS A	0.8	6.0	0.30	0.59	0.30	51.7
12	R2	115	2.0	0.192	7.4	LOS A	0.8	6.0	0.30	0.59	0.30	50.5
Approa	ach	200	2.9	0.192	6.8	LOS A	0.8	6.0	0.30	0.59	0.30	51.0
All Veh	nicles	522	5.0	0.192	4.6	NA	0.8	6.0	0.16	0.42	0.16	53.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## ablaSite: 1 [Peak Hour (Sunday) Pacific Hwy / Old Pacific Hwy - 2030 with Dev.]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Movement Performance - Vehicles												
Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Turri	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South: Pacific Hwy (S)												
1	L2	113	2.0	0.154	5.6	LOS A	0.0	0.0	0.00	0.23	0.00	55.5
2	T1	179	2.0	0.154	0.0	LOS A	0.0	0.0	0.00	0.23	0.00	57.9
Approa	ach	292	2.0	0.154	2.2	NA	0.0	0.0	0.00	0.23	0.00	57.1
North:	North: Pacific Hwy (N)											
8	T1	209	2.0	0.110	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	60.0
9	R2	46	2.0	0.034	6.5	LOS A	0.1	1.1	0.38	0.59	0.38	50.5
Approa	ach	256	2.0	0.110	1.2	NA	0.1	1.1	0.07	0.11	0.07	58.3
West:	West: Old Pacific Hwy											
10	L2	73	2.0	0.444	7.5	LOS A	2.8	20.0	0.55	0.84	0.75	48.7
12	R2	257	2.0	0.444	11.3	LOS A	2.8	20.0	0.55	0.84	0.75	47.6
Approa	ach	329	2.0	0.444	10.4	LOS A	2.8	20.0	0.55	0.84	0.75	47.8
All Veh	nicles	877	2.0	0.444	5.0	NA	2.8	20.0	0.23	0.42	0.30	53.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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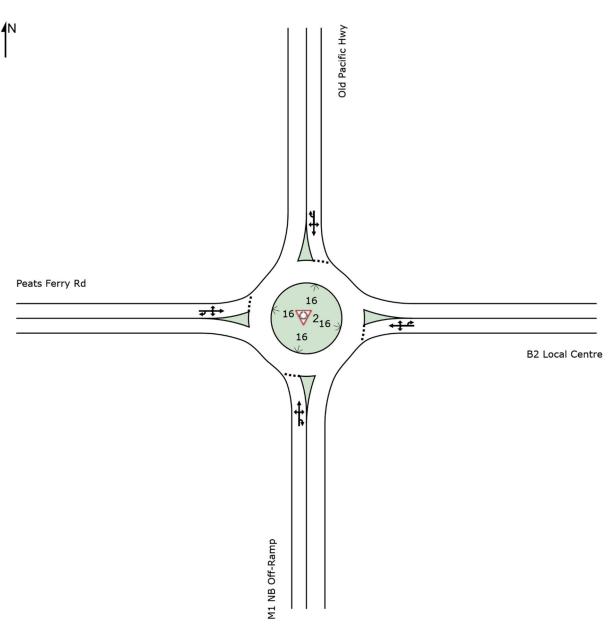
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## SITE LAYOUT

# Site: 2 Peats Ferry Rd / Pacific Hwy

New Site Site Category: (None) Roundabout



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Organisation: MOTT MACDONALD | Created: Monday, 28 September 2020 09:32:10

# Site: 2 [AM (Weekday) Peats Ferry Rd / Pacific Hwy - 2018]

New Site Site Category: (None) Roundabout

Movement Performance - Vehicles												
Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Tun	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South: M1 NB Off-Ramp												
1	L2	24	10.0	0.053	4.3	LOS A	0.2	1.9	0.09	0.45	0.09	53.7
2	T1	45	10.0	0.053	4.5	LOS A	0.2	1.9	0.09	0.45	0.09	54.5
3	R2	1	10.0	0.053	8.6	LOS A	0.2	1.9	0.09	0.45	0.09	40.2
3u	U	1	10.0	0.053	10.6	LOS A	0.2	1.9	0.09	0.45	0.09	56.0
Appro	ach	72	10.0	0.053	4.6	LOS A	0.2	1.9	0.09	0.45	0.09	54.0
East: B2 Local Centre												
4	L2	4	10.0	0.006	2.1	LOS A	0.0	0.2	0.15	0.50	0.15	51.5
5	T1	1	10.0	0.006	2.6	LOS A	0.0	0.2	0.15	0.50	0.15	52.5
6	R2	1	10.0	0.006	6.2	LOS A	0.0	0.2	0.15	0.50	0.15	47.5
6u	U	1	10.0	0.006	8.2	LOS A	0.0	0.2	0.15	0.50	0.15	12.9
Appro	ach	7	10.0	0.006	3.6	LOS A	0.0	0.2	0.15	0.50	0.15	47.3
North	Old Pa	acific Hwy										
7	L2	3	10.0	0.022	4.3	LOS A	0.1	0.8	0.09	0.55	0.09	30.6
8	T1	11	10.0	0.022	4.5	LOS A	0.1	0.8	0.09	0.55	0.09	52.7
9	R2	13	10.0	0.022	8.6	LOS A	0.1	0.8	0.09	0.55	0.09	51.4
9u	U	1	10.0	0.022	10.6	LOS A	0.1	0.8	0.09	0.55	0.09	50.0
Appro	ach	27	10.0	0.022	6.6	LOS A	0.1	0.8	0.09	0.55	0.09	49.9
West:	Peats	Ferry Rd										
10	L2	6	10.0	0.015	4.4	LOS A	0.1	0.5	0.16	0.57	0.16	49.6
11	T1	1	10.0	0.015	4.6	LOS A	0.1	0.5	0.16	0.57	0.16	35.4
12	R2	12	10.0	0.015	8.8	LOS A	0.1	0.5	0.16	0.57	0.16	52.4
12u	U	1	10.0	0.015	10.7	LOS A	0.1	0.5	0.16	0.57	0.16	52.5
Appro	Approach 20		10.0	0.015	7.3	LOS A	0.1	0.5	0.16	0.57	0.16	50.9
All Ve	hicles	126	10.0	0.053	5.4	LOS A	0.2	1.9	0.11	0.49	0.11	52.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Organisation: MOTT MACDONALD | Processed: Monday, 28 September 2020 09:16:22

# Site: 2 [PM (Weekday) Peats Ferry Rd / Pacific Hwy - 2018]

New Site Site Category: (None) Roundabout

Move	ment l	Performan	ce - \	Vehicl	es							
Mov	Turn	Demand F	lows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Turri	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South	: M1 NI	B Off-Ramp										
1	L2	35	4.0	0.075	4.2	LOS A	0.4	2.7	0.09	0.49	0.09	53.4
2	T1	58	4.0	0.075	4.4	LOS A	0.4	2.7	0.09	0.49	0.09	54.1
3	R2	1	4.0	0.075	8.6	LOS A	0.4	2.7	0.09	0.49	0.09	39.8
3u	U	14	4.0	0.075	10.5	LOS A	0.4	2.7	0.09	0.49	0.09	55.8
Appro	ach	107	4.0	0.075	5.2	LOS A	0.4	2.7	0.09	0.49	0.09	54.0
East:	B2 Loc	al Centre										
4	L2	1	4.0	0.004	2.6	LOS A	0.0	0.1	0.29	0.54	0.29	50.0
5	T1	1	4.0	0.004	3.1	LOS A	0.0	0.1	0.29	0.54	0.29	50.4
6	R2	1	4.0	0.004	6.7	LOS A	0.0	0.1	0.29	0.54	0.29	46.9
6u	U	1	4.0	0.004	8.6	LOS A	0.0	0.1	0.29	0.54	0.29	13.2
Appro	ach	4	4.0	0.004	5.3	LOS A	0.0	0.1	0.29	0.54	0.29	42.0
North:	Old Pa	acific Hwy										
7	L2	1	4.0	0.067	4.5	LOS A	0.3	2.3	0.20	0.47	0.20	31.1
8	T1	65	4.0	0.067	4.7	LOS A	0.3	2.3	0.20	0.47	0.20	53.6
9	R2	11	4.0	0.067	8.8	LOS A	0.3	2.3	0.20	0.47	0.20	52.5
9u	U	2	4.0	0.067	10.8	LOS A	0.3	2.3	0.20	0.47	0.20	52.0
Appro	ach	79	4.0	0.067	5.4	LOS A	0.3	2.3	0.20	0.47	0.20	53.2
West:	Peats	Ferry Rd										
10	L2	12	4.0	0.043	4.4	LOS A	0.2	1.5	0.20	0.59	0.20	49.1
11	T1	1	4.0	0.043	4.7	LOS A	0.2	1.5	0.20	0.59	0.20	35.0
12	R2	45	4.0	0.043	8.8	LOS A	0.2	1.5	0.20	0.59	0.20	52.1
12u	U	1	4.0	0.043	10.7	LOS A	0.2	1.5	0.20	0.59	0.20	52.2
Appro	ach	59	4.0	0.043	7.9	LOS A	0.2	1.5	0.20	0.59	0.20	51.3
All Ve	nicles	249	4.0	0.075	5.9	LOS A	0.4	2.7	0.15	0.51	0.15	52.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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# Site: 2 [Peak Hour (Sunday) Peats Ferry Rd / Pacific Hwy - 2018]

New Site Site Category: (None) Roundabout

Move	ment	Performan	ce - V	/ehicl	es							
Mov	Turn	Demand F	lows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Turri	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South	: M1 N	B Off-Ramp										
1	L2	63	2.0 (	0.161	4.2	LOS A	0.9	6.2	0.12	0.44	0.12	53.8
2	T1	167	2.0 (	0.161	4.5	LOS A	0.9	6.2	0.12	0.44	0.12	54.6
3	R2	1	2.0 (	0.161	8.6	LOS A	0.9	6.2	0.12	0.44	0.12	40.1
3u	U	5	2.0 (	0.161	10.5	LOS A	0.9	6.2	0.12	0.44	0.12	56.2
Appro	ach	237	2.0	0.161	4.5	LOS A	0.9	6.2	0.12	0.44	0.12	54.4
East:	B2 Loc	al Centre										
4	L2	2	2.0 (	0.005	2.7	LOS A	0.0	0.2	0.31	0.53	0.31	50.6
5	T1	1	2.0 (	0.005	3.2	LOS A	0.0	0.2	0.31	0.53	0.31	51.1
6	R2	1	2.0 (	0.005	6.8	LOS A	0.0	0.2	0.31	0.53	0.31	48.2
6u	U	1	2.0 (	0.005	8.7	LOS A	0.0	0.2	0.31	0.53	0.31	13.0
Appro	ach	5	2.0	0.005	4.8	LOS A	0.0	0.2	0.31	0.53	0.31	44.5
North:	Old Pa	acific Hwy										
7	L2	2	2.0 (	0.077	4.5	LOS A	0.4	2.8	0.23	0.49	0.23	30.9
8	T1	67	2.0 (	0.077	4.7	LOS A	0.4	2.8	0.23	0.49	0.23	53.3
9	R2	20	2.0 (	0.077	8.9	LOS A	0.4	2.8	0.23	0.49	0.23	52.2
9u	U	1	2.0 (	0.077	10.8	LOS A	0.4	2.8	0.23	0.49	0.23	52.1
Appro	ach	91	2.0	0.077	5.7	LOS A	0.4	2.8	0.23	0.49	0.23	52.7
West:	Peats	Ferry Rd										
10	L2	29	2.0 (	0.074	4.8	LOS A	0.4	2.6	0.32	0.60	0.32	49.2
11	T1	1	2.0 (	0.074	5.1	LOS A	0.4	2.6	0.32	0.60	0.32	34.9
12	R2	64	2.0 (	0.074	9.2	LOS A	0.4	2.6	0.32	0.60	0.32	52.1
12u	U	1	2.0 (	0.074	11.1	LOS A	0.4	2.6	0.32	0.60	0.32	52.2
Appro	ach	96	2.0	0.074	7.8	LOS A	0.4	2.6	0.32	0.60	0.32	51.2
All Ve	hicles	428	2.0	0.161	5.5	LOS A	0.9	6.2	0.19	0.49	0.19	53.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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# Site: 2 [AM (Weekday) Peats Ferry Rd / Pacific Hwy - 2030 without Dev.]

New Site Site Category: (None) Roundabout

Move	ment	Performa	nce - \	Vehicl	es							
Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Turri	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South	: M1 N	B Off-Ram	р									
1	L2	27	10.0	0.060	4.3	LOS A	0.3	2.2	0.10	0.45	0.10	53.6
2	T1	52	10.0	0.060	4.5	LOS A	0.3	2.2	0.10	0.45	0.10	54.5
3	R2	1	10.0	0.060	8.7	LOS A	0.3	2.2	0.10	0.45	0.10	40.2
3u	U	1	10.0	0.060	10.6	LOS A	0.3	2.2	0.10	0.45	0.10	56.0
Appro	ach	81	10.0	0.060	4.6	LOS A	0.3	2.2	0.10	0.45	0.10	54.0
East:	B2 Loc	al Centre										
4	L2	5	10.0	0.007	2.2	LOS A	0.0	0.3	0.18	0.49	0.18	51.6
5	T1	1	10.0	0.007	2.7	LOS A	0.0	0.3	0.18	0.49	0.18	52.6
6	R2	1	10.0	0.007	6.3	LOS A	0.0	0.3	0.18	0.49	0.18	47.6
6u	U	1	10.0	0.007	8.2	LOS A	0.0	0.3	0.18	0.49	0.18	12.8
Appro	ach	8	10.0	0.007	3.6	LOS A	0.0	0.3	0.18	0.49	0.18	48.0
North:	Old Pa	acific Hwy										
7	L2	3	10.0	0.034	4.3	LOS A	0.2	1.2	0.10	0.52	0.10	31.0
8	T1	24	10.0	0.034	4.5	LOS A	0.2	1.2	0.10	0.52	0.10	53.1
9	R2	15	10.0	0.034	8.7	LOS A	0.2	1.2	0.10	0.52	0.10	51.9
9u	U	1	10.0	0.034	10.6	LOS A	0.2	1.2	0.10	0.52	0.10	50.5
Appro	ach	43	10.0	0.034	6.1	LOS A	0.2	1.2	0.10	0.52	0.10	51.4
West:	Peats	Ferry Rd										
10	L2	7	10.0	0.016	4.4	LOS A	0.1	0.6	0.17	0.57	0.17	49.6
11	T1	1	10.0	0.016	4.6	LOS A	0.1	0.6	0.17	0.57	0.17	35.4
12	R2	13	10.0	0.016	8.8	LOS A	0.1	0.6	0.17	0.57	0.17	52.4
12u	U	1	10.0	0.016	10.7	LOS A	0.1	0.6	0.17	0.57	0.17	52.5
Appro	ach	22	10.0	0.016	7.2	LOS A	0.1	0.6	0.17	0.57	0.17	50.9
All Ve	hicles	155	10.0	0.060	5.3	LOS A	0.3	2.2	0.11	0.49	0.11	52.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Organisation: MOTT MACDONALD | Processed: Monday, 28 September 2020 09:59:37

# Site: 2 [PM (Weekday) Peats Ferry Rd / Pacific Hwy - 2030 without Dev.]

New Site Site Category: (None) Roundabout

Move	ment	Performan	ce - \	/ehicl	es							
Mov	Turn	Demand Fl	lows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	rum	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South	: M1 N	B Off-Ramp										
1	L2	39	4.0	0.084	4.2	LOS A	0.4	3.0	0.09	0.49	0.09	53.4
2	T1	65	4.0	0.084	4.4	LOS A	0.4	3.0	0.09	0.49	0.09	54.1
3	R2	1	4.0	0.084	8.6	LOS A	0.4	3.0	0.09	0.49	0.09	39.8
3u	U	16	4.0	0.084	10.5	LOS A	0.4	3.0	0.09	0.49	0.09	55.7
Appro	ach	121	4.0	0.084	5.2	LOS A	0.4	3.0	0.09	0.49	0.09	54.0
East:	B2 Loc	al Centre										
4	L2	1	4.0	0.004	2.7	LOS A	0.0	0.1	0.31	0.55	0.31	49.9
5	T1	1	4.0	0.004	3.2	LOS A	0.0	0.1	0.31	0.55	0.31	50.3
6	R2	1	4.0	0.004	6.8	LOS A	0.0	0.1	0.31	0.55	0.31	46.7
6u	U	1	4.0	0.004	8.7	LOS A	0.0	0.1	0.31	0.55	0.31	13.2
Appro	ach	4	4.0	0.004	5.4	LOS A	0.0	0.1	0.31	0.55	0.31	41.8
North:	Old Pa	acific Hwy										
7	L2	1	4.0	0.075	4.5	LOS A	0.4	2.7	0.22	0.47	0.22	31.0
8	T1	74	4.0	0.075	4.8	LOS A	0.4	2.7	0.22	0.47	0.22	53.5
9	R2	12	4.0	0.075	8.9	LOS A	0.4	2.7	0.22	0.47	0.22	52.4
9u	U	2	4.0	0.075	10.8	LOS A	0.4	2.7	0.22	0.47	0.22	52.0
Appro	ach	88	4.0	0.075	5.4	LOS A	0.4	2.7	0.22	0.47	0.22	53.1
West:	Peats	Ferry Rd										
10	L2	13	4.0	0.048	4.5	LOS A	0.2	1.7	0.21	0.59	0.21	49.1
11	T1	1	4.0	0.048	4.7	LOS A	0.2	1.7	0.21	0.59	0.21	34.9
12	R2	52	4.0	0.048	8.8	LOS A	0.2	1.7	0.21	0.59	0.21	52.0
12u	U	1	4.0	0.048	10.8	LOS A	0.2	1.7	0.21	0.59	0.21	52.1
Appro	ach	66	4.0	0.048	8.0	LOS A	0.2	1.7	0.21	0.59	0.21	51.3
All Ve	hicles	280	4.0	0.084	5.9	LOS A	0.4	3.0	0.16	0.51	0.16	52.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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# Site: 2 [Peak Hour (Sunday) Peats Ferry Rd / Pacific Hwy - 2030 without Dev.]

New Site Site Category: (None) Roundabout

Move	ment	Performan	ce - \	Vehicl	es							
Mov	Turn	Demand F	lows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Turri	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South	: M1 NI	B Off-Ramp										
1	L2	72	2.0	0.183	4.2	LOS A	1.0	7.3	0.13	0.44	0.13	53.8
2	T1	189	2.0	0.183	4.5	LOS A	1.0	7.3	0.13	0.44	0.13	54.5
3	R2	1	2.0	0.183	8.6	LOS A	1.0	7.3	0.13	0.44	0.13	40.1
Зu	U	6	2.0	0.183	10.5	LOS A	1.0	7.3	0.13	0.44	0.13	56.2
Appro	ach	268	2.0	0.183	4.6	LOS A	1.0	7.3	0.13	0.44	0.13	54.3
East:	B2 Loc	al Centre										
4	L2	2	2.0	0.005	2.8	LOS A	0.0	0.2	0.34	0.53	0.34	50.5
5	T1	1	2.0	0.005	3.3	LOS A	0.0	0.2	0.34	0.53	0.34	51.0
6	R2	1	2.0	0.005	6.9	LOS A	0.0	0.2	0.34	0.53	0.34	48.0
6u	U	1	2.0	0.005	8.8	LOS A	0.0	0.2	0.34	0.53	0.34	12.9
Appro	ach	5	2.0	0.005	4.9	LOS A	0.0	0.2	0.34	0.53	0.34	44.4
North:	Old Pa	acific Hwy										
7	L2	2	2.0	0.088	4.6	LOS A	0.5	3.2	0.25	0.49	0.25	30.8
8	T1	77	2.0	0.088	4.8	LOS A	0.5	3.2	0.25	0.49	0.25	53.2
9	R2	23	2.0	0.088	8.9	LOS A	0.5	3.2	0.25	0.49	0.25	52.1
9u	U	1	2.0	0.088	10.8	LOS A	0.5	3.2	0.25	0.49	0.25	51.9
Appro	ach	103	2.0	0.088	5.8	LOS A	0.5	3.2	0.25	0.49	0.25	52.6
West:	Peats	Ferry Rd										
10	L2	34	2.0	0.085	4.9	LOS A	0.4	3.0	0.35	0.61	0.35	49.1
11	T1	1	2.0	0.085	5.2	LOS A	0.4	3.0	0.35	0.61	0.35	34.8
12	R2	73	2.0	0.085	9.3	LOS A	0.4	3.0	0.35	0.61	0.35	52.1
12u	U	1	2.0	0.085	11.2	LOS A	0.4	3.0	0.35	0.61	0.35	52.2
Appro	ach	108	2.0	0.085	7.9	LOS A	0.4	3.0	0.35	0.61	0.35	51.1
All Ve	hicles	485	2.0	0.183	5.6	LOS A	1.0	7.3	0.21	0.49	0.21	53.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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# Site: 2 [AM (Weekday) Peats Ferry Rd / Pacific Hwy - 2030 with Dev.]

New Site Site Category: (None) Roundabout

Move	ment	Performa	nce - '	Vehicl	es							
Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	rum	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South	: M1 N	B Off-Ram	р									
1	L2	38	10.0	0.095	4.3	LOS A	0.5	3.6	0.13	0.44	0.13	53.5
2	T1	87	10.0	0.095	4.6	LOS A	0.5	3.6	0.13	0.44	0.13	54.3
3	R2	1	10.0	0.095	8.7	LOS A	0.5	3.6	0.13	0.44	0.13	40.1
3u	U	1	10.0	0.095	10.6	LOS A	0.5	3.6	0.13	0.44	0.13	55.9
Appro	ach	127	10.0	0.095	4.6	LOS A	0.5	3.6	0.13	0.44	0.13	54.0
East:	B2 Loc	al Centre										
4	L2	5	10.0	800.0	2.5	LOS A	0.0	0.3	0.27	0.50	0.27	51.3
5	T1	1	10.0	800.0	3.0	LOS A	0.0	0.3	0.27	0.50	0.27	52.2
6	R2	1	10.0	800.0	6.6	LOS A	0.0	0.3	0.27	0.50	0.27	47.2
6u	U	1	10.0	800.0	8.6	LOS A	0.0	0.3	0.27	0.50	0.27	12.6
Appro	ach	8	10.0	0.008	3.9	LOS A	0.0	0.3	0.27	0.50	0.27	47.7
North:	Old Pa	acific Hwy										
7	L2	3	10.0	0.070	4.3	LOS A	0.3	2.6	0.13	0.50	0.13	31.0
8	T1	59	10.0	0.070	4.6	LOS A	0.3	2.6	0.13	0.50	0.13	53.2
9	R2	25	10.0	0.070	8.7	LOS A	0.3	2.6	0.13	0.50	0.13	52.0
9u	U	1	10.0	0.070	10.6	LOS A	0.3	2.6	0.13	0.50	0.13	50.6
Appro	ach	88	10.0	0.070	5.8	LOS A	0.3	2.6	0.13	0.50	0.13	52.3
West:	Peats	Ferry Rd										
10	L2	45	10.0	0.053	4.6	LOS A	0.3	2.0	0.23	0.54	0.23	50.5
11	T1	1	10.0	0.053	4.8	LOS A	0.3	2.0	0.23	0.54	0.23	35.9
12	R2	23	10.0	0.053	9.0	LOS A	0.3	2.0	0.23	0.54	0.23	53.2
12u	U	1	10.0	0.053	10.9	LOS A	0.3	2.0	0.23	0.54	0.23	53.5
Appro	ach	71	10.0	0.053	6.1	LOS A	0.3	2.0	0.23	0.54	0.23	51.4
All Ve	hicles	295	10.0	0.095	5.3	LOS A	0.5	3.6	0.16	0.48	0.16	52.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## Site: 2 [PM (Weekday) Peats Ferry Rd / Pacific Hwy - 2030 with Dev.]

New Site Site Category: (None) Roundabout

Move	ment	Performan	ce - V	ehicle	es							
Mov	T	Demand F	lows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Turn	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South	: M1 N	B Off-Ramp										
1	L2	63	4.0 (	0.166	4.3	LOS A	0.9	6.5	0.17	0.46	0.17	53.3
2	T1	149	4.0 (	0.166	4.6	LOS A	0.9	6.5	0.17	0.46	0.17	54.0
3	R2	1	4.0 (	0.166	8.7	LOS A	0.9	6.5	0.17	0.46	0.17	39.7
3u	U	16	4.0 0	0.166	10.6	LOS A	0.9	6.5	0.17	0.46	0.17	55.7
Appro	ach	229	4.0 (	0.166	4.9	LOS A	0.9	6.5	0.17	0.46	0.17	53.9
East:	B2 Loc	al Centre										
4	L2	1	4.0 0	0.004	3.0	LOS A	0.0	0.1	0.36	0.55	0.36	49.6
5	T1	1	4.0 0	0.004	3.5	LOS A	0.0	0.1	0.36	0.55	0.36	49.9
6	R2	1	4.0 0	0.004	7.1	LOS A	0.0	0.1	0.36	0.55	0.36	46.4
6u	U	1	4.0 0	0.004	9.0	LOS A	0.0	0.1	0.36	0.55	0.36	13.1
Appro	ach	4	4.0 (	0.004	5.6	LOS A	0.0	0.1	0.36	0.55	0.36	41.5
North:	Old Pa	acific Hwy										
7	L2	1	4.0 0	0.111	4.5	LOS A	0.6	4.2	0.24	0.51	0.24	30.7
8	T1	93	4.0 0	0.111	4.8	LOS A	0.6	4.2	0.24	0.51	0.24	53.0
9	R2	37	4.0 0	0.111	8.9	LOS A	0.6	4.2	0.24	0.51	0.24	51.8
9u	U	2	4.0 0	0.111	10.8	LOS A	0.6	4.2	0.24	0.51	0.24	51.3
Appro	ach	133	4.0 (	0.111	6.0	LOS A	0.6	4.2	0.24	0.51	0.24	52.5
West:	Peats	Ferry Rd										
10	L2	31	4.0 0	0.069	4.8	LOS A	0.3	2.5	0.32	0.59	0.32	49.3
11	T1	1	4.0 0	0.069	5.1	LOS A	0.3	2.5	0.32	0.59	0.32	35.0
12	R2	56	4.0 0	0.069	9.2	LOS A	0.3	2.5	0.32	0.59	0.32	52.2
12u	U	1	4.0 0	0.069	11.1	LOS A	0.3	2.5	0.32	0.59	0.32	52.3
Appro	ach	88	4.0 (	0.069	7.7	LOS A	0.3	2.5	0.32	0.59	0.32	51.1
All Ve	nicles	455	4.0 (	0.166	5.8	LOS A	0.9	6.5	0.22	0.50	0.22	52.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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# Site: 2 [Peak Hour (Sunday) Peats Ferry Rd / Pacific Hwy - 2030 with Dev.]

New Site Site Category: (None) Roundabout

Move	ment	Performan	ce - '	Vehicl	es							
Mov	Turn	Demand F	lows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Turri	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South	: M1 NI	B Off-Ramp										
1	L2	84	2.0	0.262	4.9	LOS A	1.6	11.3	0.37	0.51	0.37	52.8
2	T1	225	2.0	0.262	5.2	LOS A	1.6	11.3	0.37	0.51	0.37	53.4
3	R2	1	2.0	0.262	9.3	LOS A	1.6	11.3	0.37	0.51	0.37	39.1
3u	U	6	2.0	0.262	11.2	LOS A	1.6	11.3	0.37	0.51	0.37	55.2
Appro	ach	317	2.0	0.262	5.3	LOS A	1.6	11.3	0.37	0.51	0.37	53.2
East:	B2 Loc	al Centre										
4	L2	31	2.0	0.137	3.4	LOS A	0.7	5.0	0.43	0.65	0.43	48.7
5	T1	1	2.0	0.137	3.9	LOS A	0.7	5.0	0.43	0.65	0.43	48.8
6	R2	104	2.0	0.137	7.5	LOS A	0.7	5.0	0.43	0.65	0.43	45.6
6u	U	1	2.0	0.137	9.4	LOS A	0.7	5.0	0.43	0.65	0.43	12.7
Appro	ach	137	2.0	0.137	6.5	LOS A	0.7	5.0	0.43	0.65	0.43	46.3
North:	Old Pa	acific Hwy										
7	L2	2	2.0	0.128	4.6	LOS A	0.7	5.0	0.28	0.50	0.28	30.7
8	T1	112	2.0	0.128	4.9	LOS A	0.7	5.0	0.28	0.50	0.28	53.0
9	R2	36	2.0	0.128	9.0	LOS A	0.7	5.0	0.28	0.50	0.28	51.9
9u	U	1	2.0	0.128	10.9	LOS A	0.7	5.0	0.28	0.50	0.28	51.7
Appro	ach	151	2.0	0.128	5.9	LOS A	0.7	5.0	0.28	0.50	0.28	52.5
West:	Peats	Ferry Rd										
10	L2	74	2.0	0.138	5.6	LOS A	0.8	5.5	0.48	0.65	0.48	49.2
11	T1	1	2.0	0.138	5.9	LOS A	0.8	5.5	0.48	0.65	0.48	34.8
12	R2	83	2.0	0.138	10.0	LOS A	0.8	5.5	0.48	0.65	0.48	52.2
12u	U	1	2.0	0.138	11.9	LOS A	0.8	5.5	0.48	0.65	0.48	52.3
Appro	ach	159	2.0	0.138	8.0	LOS A	0.8	5.5	0.48	0.65	0.48	50.8
All Vel	hicles	763	2.0	0.262	6.2	LOS A	1.6	11.3	0.39	0.56	0.39	51.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

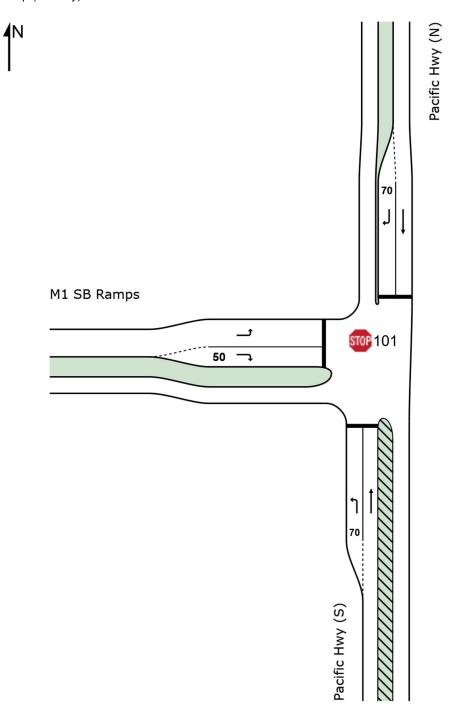
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#### SITE LAYOUT

## Site: 101 Pacific Hwy / M1 SB Ramps

New Site Site Category: (None) Stop (All-Way)



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## Site: 101 [AM (Weekday) Pacific Hwy / M1 SB Ramps - 2018]

New Site Site Category: (None) Stop (All-Way)

Move	ment	Performa	ince - '	Vehicl	es							
Mov ID	Turn	Demand Total	Flows HV	Deg. Satn	Average Delay		95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed
		veh/h	%	v/c	sec		veh	m				km/h
South:	Pacifi	c Hwy (S)										
1	L2	42	10.0	0.164	16.6	LOS B	0.6	4.5	0.97	1.28	2.16	40.6
2	T1	35	10.0	0.151	16.7	LOS B	0.5	4.1	0.98	1.28	2.15	46.2
Approa	ach	77	10.0	0.164	16.6	LOS B	0.6	4.5	0.97	1.28	2.16	43.5
North:	Pacific	Hwy (N)										
8	T1	56	10.0	0.143	12.8	LOS A	0.5	3.7	0.89	1.28	2.05	48.6
9	R2	45	10.0	0.127	12.8	LOS A	0.4	3.3	0.91	1.27	2.05	41.4
Approa	ach	101	10.0	0.143	12.8	LOS A	0.5	3.7	0.90	1.27	2.05	46.2
West:	M1 SB	Ramps										
10	L2	20	10.0	0.064	12.1	LOS A	0.2	1.6	0.92	1.25	1.99	42.5
12	R2	55	10.0	0.158	13.0	LOS A	0.6	4.2	0.92	1.28	2.10	43.4
Approa	ach	75	10.0	0.158	12.8	LOS A	0.6	4.2	0.92	1.27	2.07	43.2
All Veh	nicles	253	10.0	0.164	14.0	LOS A	0.6	4.5	0.93	1.28	2.09	44.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: C:\Users\MUL92843\Mott MacDonald\Mooney Mooney Planning Proposa - Develop\Transport\8. Update Sept 2020\SIDRA Analysis\181210 - Existing (2018).sip8

### Site: 101 [PM (Weekday) Pacific Hwy / M1 SB Ramps - 2018]

New Site Site Category: (None) Stop (All-Way)

Move	ment	Performa	ince - '	Vehicl	es							
Mov	Turn	Demand	Flows	Deg.		Level of	95% Back	of Queue	Prop.		Aver. No.	0
ID	1 ann	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South:	Pacifi	c Hwy (S)										
1	L2	36	10.0	0.110	13.1	LOS A	0.4	2.9	0.92	1.27	2.04	43.4
2	T1	81	10.0	0.228	14.7	LOS B	0.8	6.4	0.92	1.30	2.21	47.4
Approa	ach	117	10.0	0.228	14.2	LOS A	0.8	6.4	0.92	1.29	2.16	46.4
North:	Pacific	Hwy (N)										
8	T1	68	10.0	0.158	12.4	LOS A	0.5	4.1	0.87	1.28	2.05	48.9
9	R2	37	10.0	0.094	11.8	LOS A	0.3	2.4	0.88	1.26	1.98	42.4
Approa	ach	105	10.0	0.158	12.2	LOS A	0.5	4.1	0.87	1.27	2.03	47.3
West:	M1 SB	Ramps										
10	L2	17	10.0	0.066	13.5	LOS A	0.2	1.7	0.95	1.25	2.02	41.3
12	R2	31	10.0	0.105	13.6	LOS A	0.4	2.7	0.94	1.26	2.05	42.9
Approa	ach	47	10.0	0.105	13.6	LOS A	0.4	2.7	0.94	1.26	2.04	42.4
All Veh	nicles	269	10.0	0.228	13.3	LOS A	0.8	6.4	0.91	1.28	2.09	46.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: C:\Users\MUL92843\Mott MacDonald\Mooney Mooney Planning Proposa - Develop\Transport\8. Update Sept 2020\SIDRA Analysis\181210 - Existing (2018).sip8

## Site: 101 [Peak Hour (Sunday) Pacific Hwy / M1 SB Ramps - 2018]

New Site Site Category: (None) Stop (All-Way)

Move	ment	Performa	nce - '	Vehicl	es							
Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Tun	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South:	Pacifi	c Hwy (S)										
1	L2	99	5.0	0.278	14.9	LOS B	1.1	7.9	0.93	1.32	2.31	41.8
2	T1	157	5.0	0.400	17.1	LOS B	1.7	12.6	0.94	1.38	2.59	45.9
Approa	ach	256	5.0	0.400	16.3	LOS B	1.7	12.6	0.94	1.36	2.48	44.6
North:	Pacific	c Hwy (N)										
8	T1	245	5.0	0.540	19.5	LOS B	2.8	20.3	0.95	1.49	3.08	44.4
9	R2	34	5.0	0.081	11.2	LOS A	0.3	1.9	0.86	1.25	1.95	42.9
Approa	ach	279	5.0	0.540	18.5	LOS B	2.8	20.3	0.94	1.46	2.94	44.3
West:	M1 SB	8 Ramps										
10	L2	60	5.0	0.201	14.4	LOS A	0.7	5.4	0.95	1.29	2.19	40.5
12	R2	88	5.0	0.261	14.8	LOS B	1.0	7.3	0.94	1.31	2.28	42.0
Approa	ach	148	5.0	0.261	14.7	LOS B	1.0	7.3	0.94	1.30	2.25	41.5
All Veh	nicles	683	5.0	0.540	16.8	LOS B	2.8	20.3	0.94	1.39	2.62	43.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Organisation: MOTT MACDONALD | Processed: Monday, 28 September 2020 09:16:24

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## Site: 101 [AM (Weekday) Pacific Hwy / M1 SB Ramps - 2030 without Dev.]

New Site Site Category: (None) Stop (All-Way)

Move	ment	Performa	nce - '	Vehicl	es							
Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Tunn	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South:	Pacifi	c Hwy (S)										
1	L2	47	10.0	0.185	17.0	LOS B	0.7	5.1	0.97	1.29	2.19	40.3
2	T1	39	10.0	0.170	17.1	LOS B	0.6	4.7	0.98	1.28	2.18	45.9
Approa	ach	86	10.0	0.185	17.0	LOS B	0.7	5.1	0.97	1.29	2.19	43.2
North:	Pacific	: Hwy (N)										
8	T1	63	10.0	0.161	13.1	LOS A	0.6	4.3	0.89	1.28	2.08	48.5
9	R2	52	10.0	0.145	13.0	LOS A	0.5	3.8	0.91	1.28	2.08	41.2
Approa	ach	115	10.0	0.161	13.0	LOS A	0.6	4.3	0.90	1.28	2.08	46.0
West:	M1 SB	Ramps										
10	L2	23	10.0	0.074	12.3	LOS A	0.2	1.9	0.92	1.26	2.00	42.4
12	R2	62	10.0	0.179	13.3	LOS A	0.6	4.8	0.92	1.29	2.13	43.2
Approa	ach	85	10.0	0.179	13.0	LOS A	0.6	4.8	0.92	1.28	2.10	43.0
All Veh	nicles	286	10.0	0.185	14.2	LOS A	0.7	5.1	0.93	1.28	2.12	44.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Organisation: MOTT MACDONALD | Processed: Monday, 28 September 2020 09:59:39

## Site: 101 [PM (Weekday) Pacific Hwy / M1 SB Ramps - 2030 without Dev.]

New Site Site Category: (None) Stop (All-Way)

Move	ment	Performa	nce - '	Vehicl	es							
Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Turr	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South:	Pacifi	c Hwy (S)										
1	L2	41	10.0	0.126	13.3	LOS A	0.4	3.3	0.92	1.27	2.06	43.2
2	T1	92	10.0	0.258	15.3	LOS B	1.0	7.4	0.93	1.32	2.27	47.0
Approa	ach	133	10.0	0.258	14.7	LOS B	1.0	7.4	0.93	1.30	2.21	46.1
North:	Pacific	Hwy (N)										
8	T1	78	10.0	0.180	12.6	LOS A	0.6	4.8	0.87	1.29	2.09	48.8
9	R2	42	10.0	0.107	11.9	LOS A	0.4	2.7	0.88	1.27	2.00	42.2
Approa	ach	120	10.0	0.180	12.4	LOS A	0.6	4.8	0.88	1.28	2.06	47.1
West:	M1 SB	Ramps										
10	L2	19	10.0	0.074	13.6	LOS A	0.3	1.9	0.95	1.25	2.03	41.2
12	R2	35	10.0	0.119	13.9	LOS A	0.4	3.1	0.94	1.27	2.07	42.7
Approa	ach	54	10.0	0.119	13.8	LOS A	0.4	3.1	0.95	1.26	2.06	42.2
All Veh	nicles	306	10.0	0.258	13.6	LOS A	1.0	7.4	0.91	1.29	2.12	45.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Organisation: MOTT MACDONALD | Processed: Monday, 28 September 2020 09:59:39

## Site: 101 [Peak Hour (Sunday) Pacific Hwy / M1 SB Ramps - 2030 without Dev.]

New Site Site Category: (None) Stop (All-Way)

Move	ment	Performa	nce - V	Vehicl	es							
Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Tunn	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South:	Pacifi	c Hwy (S)										
1	L2	113	5.0	0.316	15.7	LOS B	1.3	9.2	0.94	1.34	2.39	41.2
2	T1	178	5.0	0.453	18.5	LOS B	2.1	15.2	0.95	1.42	2.75	45.0
Approa	ach	291	5.0	0.453	17.4	LOS B	2.1	15.2	0.95	1.39	2.61	43.8
North:	Pacific	c Hwy (N)										
8	T1	278	5.0	0.611	22.4	LOS B	3.5	25.8	0.97	1.57	3.46	42.8
9	R2	38	5.0	0.091	11.3	LOS A	0.3	2.2	0.86	1.26	1.96	42.8
Approa	ach	316	5.0	0.611	21.1	LOS B	3.5	25.8	0.96	1.54	3.28	42.8
West:	M1 SB	Ramps										
10	L2	68	5.0	0.229	14.9	LOS B	0.9	6.3	0.95	1.30	2.24	40.1
12	R2	100	5.0	0.295	15.5	LOS B	1.2	8.5	0.95	1.33	2.35	41.5
Approa	ach	168	5.0	0.295	15.3	LOS B	1.2	8.5	0.95	1.32	2.31	41.0
All Veh	nicles	775	5.0	0.611	18.4	LOS B	3.5	25.8	0.95	1.43	2.82	42.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Organisation: MOTT MACDONALD | Processed: Monday, 28 September 2020 09:59:40

## Site: 101 [AM (Weekday) Pacific Hwy / M1 SB Ramps - 2030 with Dev.]

New Site Site Category: (None) Stop (All-Way)

Move	ment	Performa	nce - \	Vehicl	es							
Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Tunn	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South:	Pacifi	c Hwy (S)										
1	L2	47	10.0	0.183	16.1	LOS B	0.7	5.1	0.97	1.29	2.19	41.0
2	T1	59	10.0	0.201	16.1	LOS B	0.7	5.6	0.95	1.29	2.20	46.5
Approa	ach	106	10.0	0.201	16.1	LOS B	0.7	5.6	0.96	1.29	2.19	44.5
North:	Pacific	: Hwy (N)										
8	T1	108	10.0	0.249	13.2	LOS A	0.9	7.0	0.89	1.31	2.21	48.4
9	R2	157	10.0	0.337	14.4	LOS A	1.3	10.2	0.89	1.35	2.39	39.9
Approa	ach	265	10.0	0.337	13.9	LOS A	1.3	10.2	0.89	1.34	2.32	44.3
West:	M1 SB	Ramps										
10	L2	42	10.0	0.178	16.3	LOS B	0.7	4.9	0.98	1.29	2.19	38.8
12	R2	62	10.0	0.231	16.7	LOS B	0.9	6.7	0.97	1.31	2.26	40.4
Approa	ach	104	10.0	0.231	16.5	LOS B	0.9	6.7	0.97	1.30	2.23	39.8
All Veh	nicles	476	10.0	0.337	15.0	LOS B	1.3	10.2	0.92	1.32	2.27	43.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## Site: 101 [PM (Weekday) Pacific Hwy / M1 SB Ramps - 2030 with Dev.]

New Site Site Category: (None) Stop (All-Way)

Move	ment	Performa	nce - '	Vehicl	es							
Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Turri	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South:	Pacifi	c Hwy (S)										
1	L2	41	10.0	0.112	12.3	LOS A	0.4	2.9	0.90	1.27	2.02	44.0
2	T1	138	10.0	0.340	15.7	LOS B	1.4	10.5	0.92	1.36	2.43	46.7
Approa	ach	179	10.0	0.340	14.9	LOS B	1.4	10.5	0.92	1.33	2.33	46.3
North:	Pacific	: Hwy (N)										
8	T1	101	10.0	0.200	12.0	LOS A	0.7	5.3	0.84	1.30	2.09	49.2
9	R2	96	10.0	0.208	12.2	LOS A	0.7	5.6	0.86	1.30	2.12	41.9
Approa	ach	197	10.0	0.208	12.1	LOS A	0.7	5.6	0.85	1.30	2.10	46.4
West:	M1 SB	Ramps										
10	L2	65	10.0	0.228	16.2	LOS B	0.9	6.5	0.96	1.30	2.25	38.8
12	R2	35	10.0	0.137	14.5	LOS B	0.5	3.7	0.96	1.27	2.12	42.2
Approa	ach	100	10.0	0.228	15.6	LOS B	0.9	6.5	0.96	1.29	2.20	40.1
All Veh	nicles	476	10.0	0.340	13.9	LOS A	1.4	10.5	0.90	1.31	2.21	45.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Organisation: MOTT MACDONALD | Processed: Friday, 18 September 2020 14:41:07

## Site: 101 [Peak Hour (Sunday) Pacific Hwy / M1 SB Ramps - 2030 with Dev.]

New Site Site Category: (None) Stop (All-Way)

Move	ment	Performa	nce - \	Vehicl	es							
Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Turri	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South:	Pacifi	c Hwy (S)										
1	L2	113	5.0	0.326	16.1	LOS B	1.3	9.6	0.95	1.34	2.42	40.9
2	T1	198	5.0	0.514	20.9	LOS B	2.6	18.7	0.97	1.47	2.99	43.6
Approa	ach	311	5.0	0.514	19.2	LOS B	2.6	18.7	0.96	1.42	2.78	42.8
North:	Pacific	c Hwy (N)										
8	T1	324	5.0	0.663	24.0	LOS B	4.2	30.9	0.98	1.66	3.82	41.9
9	R2	145	5.0	0.322	13.9	LOS A	1.3	9.3	0.90	1.34	2.36	40.3
Approa	ach	469	5.0	0.663	20.8	LOS B	4.2	30.9	0.95	1.56	3.37	41.6
West:	M1 SB	Ramps										
10	L2	88	5.0	0.333	18.8	LOS B	1.4	10.0	0.99	1.34	2.47	36.9
12	R2	100	5.0	0.331	17.7	LOS B	1.4	9.9	0.97	1.34	2.45	39.9
Approa	ach	188	5.0	0.333	18.2	LOS B	1.4	10.0	0.98	1.34	2.46	38.6
All Veh	nicles	968	5.0	0.663	19.8	LOS B	4.2	30.9	0.96	1.47	3.00	41.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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# H. HCM Analysis

	N.	Peak		Merge or	No.of	Freeway		Ramp	Acceleration / Deceleration	Ramp Influe Lane 1 & 2	ence Area - within 450m	Freeway Lanes	before ramp	- All	Ramp - A	II Lanes		Freeway	after ramp -	All Lanes
No.	Year	Hour	Freeway- Ramp Terminal	Diverge Type	-	Flow (pc/h)	Ramp Lane	Flow (pc/h)	lane length/ (m)	Density /(pc/km/ln)	Level of Service	V/C	Density /(pc/km/ln)	Level of Service	V/C	Density /(pc/km/ln)	Level of Service	V/C	Density (pc/km/ln)	Level of Service
1			Pacific Highway (M1) Off-Ramp (Northbound)	Diverge	3	2038	1	77	60	8.9	В	0.29	8.86	В	0.04	1.28	Α	0.28	5.94	Α
2	2018	AM	Pacific Highway (M1) Off-Ramp (Southbound)	Diverge	3	5420	1	81	60	16.4	С	0.77	16.37	С	0.04	1.35	Α	0.76	16.18	С
3	2010	A.III	Pacific Highway (M1) On-Ramp (Northbound)	Merge	3	1966	1	43	150	7.3	В	0.28	5.96	Α	0.02	0.72	Α	0.29	7.26	В
4			Pacific Highway (M1) On-Ramp (Southbound)	Merge	3	5545	1	88	100	18.1	D	0.79	16.80	С	0.04	1.47	Α	0.80	18.14	D
1			Pacific Highway (M1) Off-Ramp (Northbound)	Diverge	3	1874	1	87	60	8.4	В	0.27	8.37	В	0.04	1.46	Α	0.25	5.41	Α
2	2030 without	AM	Pacific Highway (M1) Off-Ramp (Southbound)	Diverge	3	5320	1	123	60	16.3	С	0.75	16.30	С	0.06	2.05	Α	0.74	15.75	С
3	Development	A.III	Pacific Highway (M1) On-Ramp (Northbound)	Merge	3	2228	1	49	150	8.0	В	0.32	6.75	В	0.02	0.82	Α	0.32	8.04	В
4			Pacific Highway (M1) On-Ramp (Southbound)	Merge	3	5020	1	100	100	16.7	С	0.71	15.21	С	0.05	1.67	Α	0.73	16.72	С
1			Pacific Highway (M1) Off-Ramp (Northbound)	Diverge	3	1920	1	134	60	8.6	В	0.27	8.58	В	0.07	2.23	Α	0.25	5.41	Α
2	2030 with	AM	Pacific Highway (M1) Off-Ramp (Southbound)	Diverge	3	5340	1	112	60	16.3	С	0.76	16.31	С	0.06	1.86	Α	0.74	15.84	С
3	Development	A.III	Pacific Highway (M1) On-Ramp (Northbound)	Merge	3	2274	1	95	150	8.4	В	0.32	6.89	В	0.05	1.58	Α	0.34	8.37	В
4			Pacific Highway (M1) On-Ramp (Southbound)	Merge	3	6388	1	205	100	21.1	D	0.91	19.36	D	0.10	3.42	Α	0.94	21.05	D
1			Pacific Highway (M1) Off-Ramp (Northbound)	Diverge	3	5135	1	108	60	16.0	С	0.73	16.00	С	0.05	1.81	Α	0.71	15.23	С
2	2018	РМ	Pacific Highway (M1) Off-Ramp (Southbound)	Diverge	3	2626	1	49	60	10.5	В	0.37	10.52	В	0.02	0.82	Α	0.37	7.81	В
3	2010	1 141	Pacific Highway (M1) On-Ramp (Northbound)	Merge	3	4676	1	128	150	15.3	С	0.66	14.17	С	0.06	2.14	Α	0.68	15.34	С
4			Pacific Highway (M1) On-Ramp (Southbound)	Merge	3	2618	1	85	100	9.9	В	0.37	7.93	В	0.04	1.42	Α	0.38	9.89	В
1			Pacific Highway (M1) Off-Ramp (Northbound)	Diverge	3	5285	1	122	60	16.3	С	0.75	16.25	С	0.06	2.04	Α	0.73	15.65	С
2	2030 without	РМ	Pacific Highway (M1) Off-Ramp (Southbound)	Diverge	3	2635	1	56	60	10.6	В	0.37	10.55	В	0.03	0.93	Α	0.37	7.81	В
3	Development	I- IVI	Pacific Highway (M1) On-Ramp (Northbound)	Merge	3	5297	1	146	150	17.2	D	0.75	16.05	С	0.07	2.44	Α	0.77	17.19	D
4			Pacific Highway (M1) On-Ramp (Southbound)	Merge	3	2965	1	91	100	10.9	В	0.42	8.99	В	0.05	1.51	Α	0.43	10.89	В
1			Pacific Highway (M1) Off-Ramp (Northbound)	Diverge	3	5297	1	134	60	16.3	С	0.75	16.29	С	0.07	2.23	Α	0.73	15.65	С
2	2030 with	РМ	Pacific Highway (M1) Off-Ramp (Southbound)	Diverge	3	2635	1	102	60	10.6	В	0.37	10.63	В	0.05	1.70	Α	0.36	7.67	В
3	Development	I~ IVI	Pacific Highway (M1) On-Ramp (Northbound)	Merge	3	5320	1	169	150	17.4	D	0.75	16.12	С	0.08	2.82	Α	0.78	17.36	D
4			Pacific Highway (M1) On-Ramp (Southbound)	Merge	3	3019	1	144	100	11.3	В	0.43	9.15	В	0.07	2.40	Α	0.45	11.28	В
1			Pacific Highway (M1) Off-Ramp (Northbound)	Diverge	3	3832	1	236	60	13.8	С	0.54	13.81	С	0.12	3.93	Α	0.51	10.90	В
2	2018	Sunday	Pacific Highway (M1) Off-Ramp (Southbound)	Diverge	3	3952	1	181	60	14.0	С	0.56	13.97	С	0.09	3.02	Α	0.53	11.43	В
3	2016	Sunday	Pacific Highway (M1) On-Ramp (Northbound)	Merge	3	3102	1	141	150	10.9	В	0.44	9.40	В	0.07	2.35	Α	0.46	10.93	В
4			Pacific Highway (M1) On-Ramp (Southbound)	Merge	3	3848	1	133	100	13.6	С	0.55	11.66	В	0.07	2.21	Α	0.56	13.57	С
1			Pacific Highway (M1) Off-Ramp (Northbound)	Diverge	3	4176	1	267	60	14.6	С	0.59	14.59	С	0.13	4.46	Α	0.55	11.84	В
2	2030 without	Sunday	Pacific Highway (M1) Off-Ramp (Southbound)	Diverge	3	4258	1	201	60	14.6	С	0.60	14.63	С	0.10	3.35	Α	0.58	12.29	С
3	Development	Sunday	Pacific Highway (M1) On-Ramp (Northbound)	Merge	3	4074	1	159	150	13.8	С	0.58	12.34	С	0.08	2.65	Α	0.60	13.77	С
4			Pacific Highway (M1) On-Ramp (Southbound)	Merge	3	4277	1	182	100	15.0	С	0.61	12.96	С	0.09	3.04	Α	0.63	15.00	С
1			Pacific Highway (M1) Off-Ramp (Northbound)	Diverge	3	4224	1	315	60	14.8	С	0.60	14.77	С	0.16	5.25	Α	0.55	11.85	В
2	2030 with	Cundor	Pacific Highway (M1) Off-Ramp (Southbound)	Diverge	3	4278	1	189	60	14.6	С	0.61	14.65	С	0.09	3.16	Α	0.58	12.39	С
3	Development	Sunday	Pacific Highway (M1) On-Ramp (Northbound)	Merge	3	3561	1	204	150	12.5	С	0.51	10.79	В	0.10	3.40	Α	0.53	12.52	С
4			Pacific Highway (M1) On-Ramp (Southbound)	Merge	3	4465	1	257	100	15.9	С	0.63	13.53	С	0.13	4.28	Α	0.67	15.87	С

Freeway Off-ramp Diverge LOS Calcul						·	М
							MOTT
Project:	Mooney N	lonney/ Pac	ific Highway	1			MACDONA
Job No.:	397610						
Section Description:		ghway (M1)	Off-Ramp (N	orthbound)	-	AM 2018	):
User: Date:	J. Muller 28/09/2020	1					
Buto.	20/03/2020						
			V <sub>R</sub>	V <sub>R</sub>	66	veh/h	
V <sub>F</sub>	/		•R	V <sub>R</sub>		veh/h	
₩F				۷F	1,071	VCII/II	
					LV	HV	%HV
				V <sub>R</sub>	59	7	10.6
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>	1,206	365	23.2
FFS - Freeway		110	km/h	•			
No. of lanes on freeway segment	S <sub>FF</sub>	3	N11/11				
No. of lanes on Off-Ramp		1					
Deceleration lane length	L <sub>D</sub>	60	m				
Distance to adjacent downstream Ramp		N/A		applicable for	Equation 7	only	
	L <sub>dow n</sub>		m	applicable for	•	•	
Distance to adjacent upstream Ramp	L <sub>up</sub>	N/A	m	applicable for	Equation 6	only	
Passenger car equivalent for truck (HV factor) Proportion of through freeway flow remaining in	Et	2	Given by SWT	C. Cross check	with Exhibi	t 23-9, if gra	de is known
Proportion of through freeway flow remaining in Lanes 1 and 2 immediately upstream of							
diverge	P <sub>FD</sub>	0.664	6 Lane FW	0.664	Equation 5	5 from Exhil	oit 25-12
Intermediate speed determination variable for	• FD				- 1		
diverge area	Ds	0.410					
Space Mean Speed in Ramp Influence	-						
Area (450m)	S <sub>R</sub>	92.4	km/h	84%	of FFS		
Number of out side lane	No	1.0					
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>	658.3	<1000pc/h	>1000pc/h			
Space Mean Speed in outer lanes	S <sub>0</sub>	116.6	116.6	118.7			
	0		110.0	110.1			
Conversion to pc/h under base conditions							No.of Lane
pc/h	v	PHF	%HV	f <sub>HV</sub>	fp	v (pc/h)	
V <sub>F</sub>	1571	0.95	23.23	0.811	1	2038	
V <sub>R</sub>	66	0.95	10.61	0.904	1	77	
V <sub>12</sub>						1380	
V <sub>FO</sub>						1961	
Discourse							
Diverge	5						
Density of diverge influence area	D <sub>R</sub>		pc/km/ln				
Level of Service		B					
Capacity Check		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>			
Density as per Graph (Exhibit 11.6) of HCM							
2010 - pc/ln/ speed	Density		1.3	5.9	pc/km/ln		
Level of Service	,	В	A				
V/C		0.29					
FW/Ramp max. lane capacity based on speed		2350	2000	2350			

							М	
							MOTT	N
Project:		Ionney/ Pac	ific Highway	5			MACD	ONALI
Job No.: Section Description:	397610	aburat (M4)	Off-Ramp (S	auth haund)		AM 2018	i	
User:	J. Muller	giiway (wii)	On-Kamp (S	oumbound)		AIVI 2010	V	
Date:	28/09/2020	)						
			V <sub>R</sub>	V <sub>R</sub>	71	veh/h		
V <sub>F</sub>				V <sub>F</sub>	4,460	veh/h		
					LV		0/11/	
				v		HV 6	%HV	8.4
	0			V <sub>R</sub>				
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>	3,771	689		15.4
FFS - Freeway	S <sub>FF</sub>	110	km/h					
No. of lanes on freeway segment No. of lanes on Off-Ramp		3 1						
	1							
Deceleration lane length	L <sub>D</sub>	60	m					
Distance to adjacent downstream Ramp	L <sub>dow n</sub>	N/A	m	applicable for	Equation 7	only		
Distance to adjacent upstream Ramp	L <sub>up</sub>	N/A	m	applicable for	Equation 6	only		
Decompose one on we lost for truck (HV footor)	_	_						
Passenger car equivalent for truck (HV factor) Proportion of through freeway flow remaining in	Et	2	Given by SWT	C. Cross check	with Exhibi	t 23-9, if gra	de is kn	own
Lanes 1 and 2 immediately upstream of								
diverge	P <sub>FD</sub>	0.509	6 Lane FW	0.509	Equation 5	5 from Exhil	oit 25-1	2
Intermediate speed determination variable for	_							
diverge area	D <sub>S</sub>	0.410						
Space Mean Speed in Ramp Influence Area (450m)	S <sub>R</sub>	92.4	km/h	84%	of FFS			
Number of out side lane	No	1.0						
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>	2623.3	<1000pc/h	>1000pc/h				
Space Mean Speed in outer lanes	S <sub>0</sub>	106.5	116.6	106.5				
Conversion to pc/h under base conditions	N/	DUE	0/110/	4	6		No.of L	_ane
pc/h	V	PHF	%HV	f <sub>HV</sub>	fp	v (pc/h)		
V <sub>F</sub>	4460	0.95	15.45		1	5420		:
V <sub>R</sub>	71	0.95	8.45	0.922	1	81		
V <sub>12</sub>						2797		
V <sub>FO</sub>						5339		
Diverge								
Density of diverge influence area	D <sub>R</sub>	16.4	pc/km/ln					
Level of Service	- r.	C						
Capacity Check		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>				
Density as per Graph (Exhibit 11.6) of HCM								
2010 - pc/ln/ speed	Density		1.4	16.2	pc/km/ln			
Level of Service		С	A					
V/C		0.77						
FW/Ramp max. lane capacity based on speed		2350	2000	2350				

Freeway Off-ramp Diverge LOS Calcul	ation in a	ccordance	with Austro	ads/ HCM				
							Μ	
Project:	Mooney N	Ionney/ Pac	ific Highway				MOTT	M
Job No.:	397610							
Section Description:		ghwav (M1)	On-Ramp (N	orthbound)		AM 2018	(	
User:	J. Muller	<u></u>		,				
Date:	28/09/2020	)						
V <sub>R</sub>				V <sub>R</sub>	35	veh/h		
				V <sub>F</sub>	1478	veh/h		
V <sub>F</sub>			•					
					LV	HV	%HV	
				V <sub>R</sub>	29	6		17.14
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>	1,088	390		26.39
FFS - Freeway	S <sub>FF</sub>	110	km/h					
No. of lanes on freeway segment		3						
No. of lanes on On-Ramp		1						
Acceleration lane length	L <sub>A</sub>	150	m					
Distance to adjacent downstream Ramp	L <sub>dow n</sub>	N/A	m	applicable for	Equation 3	only		
Distance to adjacent upstream Ramp	L <sub>up</sub>	N/A	m	applicable for	Equation 2	only		
	up							
Passenger car equivalent for truck (HV factor)	E	2				o 'r 1 -		
Proportion of approaching freeway flow	Et	2	Given by SWIC	C. Cross check wi	ith Exhibit 23-	9, if grade is	know n.	
remaining in Lanes 1 and 2 immediately								
upstream of merge	P <sub>FM</sub>	0.591	6 Lane FW	0.5913	Equation 1	l		
Intermediate speed determination variable for								
merge area	M <sub>S</sub>	0.286						
Space Mean Speed in Ramp Influence Area (450m)	c	07.7	km/h	900/	of FFS			
	S <sub>R</sub>			0970	01773			
Number of out side lane	N <sub>O</sub>	1.0						
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>	803.6	<500pc/h	>500<2300pc/h	>2300pc/h	ı		
Space Mean Speed in outer lanes	So	108.2	110	108.2	114.4437			
Conversion to pc/h under base conditions			0/1 D /				No.of	Lane
pc/h	v	PHF	%HV	f <sub>HV</sub>	fp	v		
V <sub>F</sub>	1478	0.95	26.39	0.791	1	1966		3
V <sub>R</sub>	35	0.95	17.14	0.854	1	43		1
V <sub>12</sub>						1163		
V <sub>R12</sub>						1206		
V <sub>FO</sub>						2009		3
Merge						2000		
Density of merge influence area	D <sub>R</sub>	7.26	pc/km/ln					
Level of Service		B						
		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>				
Capacity Check		• ۴	- IX	• FU				
Density as per Graph (Exhibit 11.6) of HCM								
2010 - pc/ln/ speed	Density	5.96	0.72					
Level of Service		А						
		0.28						
FW/Ramp max. lane capacity based on speed		2350						

Freeway Off-ramp Diverge LOS Calcul	ation in a	ccordance	with Austro	ads/ HCM			
Project:	Mooney M	lonnev/ Pac	ific Highway				M MOTT MACDONALD
Job No.: Section Description:	397610 Pacific Hi	abway (M1)	On-Ramp (S	outhbound)		AM 2018	6
User:	J. Muller	giiway (mii)	on namp (o	outhoundy		7.111 2010	Voe
Date:	28/09/2020	)					
V <sub>R</sub>				V <sub>R</sub>	83	veh/h	
				V <sub>F</sub>	4572	veh/h	
V <sub>F</sub>							
					LV	ΗV	%HV
				V <sub>R</sub>	82	. 1	1.20
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>	3,876	696	15.22
FFS - Freeway	S <sub>FF</sub>	110	km/h				
No. of lanes on freeway segment		3					
No. of lanes on On-Ramp		1					
Acceleration lane length	L <sub>A</sub>	100	m				
Distance to adjacent downstream Ramp	L <sub>dow n</sub>	N/A	m	applicable for	Equation 3	only	
Distance to adjacent upstream Ramp	L <sub>up</sub>	N/A	m	applicable for	Equation 2	only	
	up				Equation 2	only	
Passenger car equivalent for truck (HV factor)	-	<u>_</u>					
Proportion of approaching freeway flow	Et	2	Given by SWTC	Cross check wi	ith Exhibit 23-	9, if grade is	know n.
remaining in Lanes 1 and 2 immediately							
upstream of merge	P <sub>FM</sub>	0.587	6 Lane FW	0.5867	Equation 1	I	
Intermediate speed determination variable for							
merge area	M <sub>S</sub>	0.383					
Space Mean Speed in Ramp Influence Area (450m)	<u> </u>	02.5	Lesser (he	050/	of FFS		
	S <sub>R</sub>		km/h	00%	01 FF 5		
Number of out side lane	N <sub>O</sub>	1.0					
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>	2291.9	<500pc/h	>500<2300pc/h	>2300pc/h	n	
Space Mean Speed in outer lanes	So	99.6	110	99.6	99.56143		
Conversion to pc/h under base conditions				-	-		No.of Lane
pc/h	v	PHF	%HV	f <sub>HV</sub>	fp	v	
V <sub>F</sub>	4572	0.95	15.22	0.868	1	5545	3
V <sub>R</sub>	83	0.95	1.20	0.988	1	88	1
V <sub>12</sub>						3253	
V <sub>R12</sub>						3342	
V <sub>FO</sub>						5634	3
Merge							
Density of merge influence area	D <sub>R</sub>		pc/km/ln				
Level of Service		D					
		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>			
Capacity Check							
Density as per Graph (Exhibit 11.6) of HCM							
2010 - pc/ln/ speed	Density	16.80					
Level of Service V/C		<b>C</b> 0.79					
FW/Ramp max. lane capacity based on speed		2350					

Freeway Off-ramp Diverge LOS Calcul							М
Project:	Mooney N	lonney/ Pac	ific Highway	9			MACDONA
Job No.:	397610						
Section Description:	Pacific Hi J. Muller	ghway (M1)	Off-Ramp (N	orthbound)		PM 2018	)
User: Date:	28/09/2020	<b>)</b>					
240.	20/00/2020						
			V <sub>R</sub>	V <sub>R</sub>	101	veh/h	
V <sub>F</sub>			- N	V <sub>F</sub>		veh/h	
•F				• F	.,		
					LV	ΗV	%HV
				V <sub>R</sub>	99	2	1.9
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>	3,984	447	10.0
FFS - Freeway	S <sub>FF</sub>	110	km/h				
No. of lanes on freeway segment		3					
No. of lanes on Off-Ramp		1					
Deceleration lane length	L <sub>D</sub>	60	m				
Distance to adjacent downstream Ramp	L <sub>dow n</sub>	N/A	m	applicable for	Equation 7	only	
Distance to adjacent upstream Ramp					•	•	
	L <sub>up</sub>	N/A	m	applicable for	Equation 6	σπιγ	
Passenger car equivalent for truck (HV factor)	-	_					
Proportion of through freeway flow remaining in	Et	2	Given by SWI	C. Cross check	with Exhibi	t 23-9, if gra	de is known
Lanes 1 and 2 immediately upstream of							
diverge	P <sub>FD</sub>	0.521	6 Lane FW	0.521	Equation 5	from Exhil	oit 25-12
Intermediate speed determination variable for							
diverge area	D <sub>S</sub>	0.413					
Space Mean Speed in Ramp Influence Area (450m)	<u> </u>		L	0.40/	-4550		
	S <sub>R</sub>		km/h	84%	of FFS		
Number of out side lane	No	1.0					
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>	2407.1	<1000pc/h	>1000pc/h			
Space Mean Speed in outer lanes	So	107.9	116.6	107.9			
Conversion to pc/h under base conditions							No.of Lane
pc/h	V	PHF	%HV	f <sub>HV</sub>	fp	v (pc/h)	
V <sub>F</sub>	4431	0.95	10.09	0.908	1	5135	
V <sub>R</sub>	101	0.95	1.98	0.981	1	108	
V <sub>12</sub>						2728	
V <sub>FO</sub>						5026	
▼FU						5020	
Diverge							
Density of diverge influence area		16.0	pc/km/ln				
Level of Service	D <sub>R</sub>	16.0 C	pe/km/m				
		-	V	V			
Capacity Check		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>			
Density as per Graph (Exhibit 11.6) of HCM							
2010 - pc/ln/ speed	Density		1.8	15.2	pc/km/ln		
Level of Service		С	A				
V/C FW/Ramp max. lane capacity based on speed		0.73 2350					
withamp max, lane capacity based on speed		2330	2000	2350			

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							MOTT	N
Project:		Ionney/ Pac	ific Highway	8			MACD	ONAL
Job No.: Section Description:	397610		0ff Dama (C		1	DM 2040		
User:	J. Muller	gnway (M1)	Off-Ramp (S	outribouna)		PM 2018		
Date:	28/09/2020	)						
			V <sub>R</sub>	V <sub>R</sub>	45	veh/h		
V <sub>F</sub>				VF	2,209	veh/h		
					LV	HV	%HV	
				V <sub>R</sub>	43	2		4.4
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>	1,923	286		12.9
FFS - Freeway	S <sub>FF</sub>	110	km/h					
No. of lanes on freeway segment		3						
No. of lanes on Off-Ramp		1						
Deceleration lane length	L <sub>D</sub>	60	m					
Distance to adjacent downstream Ramp	L <sub>dow n</sub>	N/A	m	applicable for	Equation 7	only		
Distance to adjacent upstream Ramp	L <sub>up</sub>	N/A	m	applicable for	Equation 6	only		
	-up					<b>y</b>		
Passenger car equivalent for truck (HV factor)	Et	2	Given by SW/T	C. Cross check	with Exhibi	t23_9 if ara	da is kn	own
Proportion of through freeway flow remaining in		2	Given by Svvi			t 25-9, il gla		OWIT
Lanes 1 and 2 immediately upstream of								
diverge	P <sub>FD</sub>	0.638	6 Lane FW	0.638	Equation 5	5 from Exhil	oit 25-1	2
Intermediate speed determination variable for	_	0.407						
diverge area	D <sub>S</sub>	0.407						
Space Mean Speed in Ramp Influence Area (450m)	S <sub>R</sub>	92.5	km/h	84%	of FFS			
Number of out side lane			K11711	0470	01110			
	No	1.0						
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>	932.9	<1000pc/h	>1000pc/h				
Space Mean Speed in outer lanes	So	116.6	116.6	117.0				
Conversion to pc/h under base conditions				-	-		No.of I	∟ane
pc/h	V	PHF	%HV	f <sub>HV</sub>	fp	v (pc/h)	-	
V <sub>F</sub>	2209	0.95	12.95	0.885	1	2626		
V <sub>R</sub>	45	0.95	4.44	0.957	1	49		
V <sub>12</sub>						1693		
V <sub>FO</sub>						2577		
Γ FU						2011		
Diverge								
Density of diverge influence area		10 5	pc/km/ln					
Level of Service	D <sub>R</sub>	10.5 B	po/km/m					
			V_	1/				
Capacity Check		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>				
Density as per Graph (Exhibit 11.6) of HCM								
2010 - pc/ln/ speed	Density		0.8	7.8	pc/km/ln			
Level of Service		B	A					
V/C FW/Ramp max. lane capacity based on speed		0.37 2350						
		2350	2000	2330				

Freeway Off-ramp Diverge LOS Calcul	ation in a	ccordance	with Austro	ads/ HCM				
							Μ	
Project:	Mooney N	lonney/ Pac	ific Highway	15			MOTT	MALD
Job No.:	397610							
Section Description:	Pacific Hi	ghway (M1)	On-Ramp (N	orthbound)		PM 2018	6	
User:	J. Muller							
Date:	28/09/2020	)	1		1	1		
V <sub>R</sub>				V <sub>R</sub>	1	veh/h		
				V <sub>F</sub>	3955	veh/h		
V <sub>F</sub>		*	•					
					LV	HV	%HV	
				V <sub>R</sub>	114	4		3.39
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>	3,468	487		12.31
FFS - Freeway	SFF	110	km/h					
No. of lanes on freeway segment		3						
No. of lanes on On-Ramp		1						
Acceleration lane length	L <sub>A</sub>	150	m					
Distance to adjacent downstream Ramp	L <sub>dow n</sub>	N/A	m	applicable for	Equation 3	only		
Distance to adjacent upstream Ramp	L <sub>up</sub>	N/A	m	applicable for	Equation 2	only		
	Lup					only		
Descention containing lant for tweak (11) (footor)	_	_						
Passenger car equivalent for truck (HV factor) Proportion of approaching freeway flow	Et	2	Given by SWTC	2. Cross check w	ith Exhibit 23-	9, if grade is	know n.	
remaining in Lanes 1 and 2 immediately								
upstream of merge	P <sub>FM</sub>	0.591	6 Lane FW	0.5913	Equation 1	I		
Intermediate speed determination variable for					•			
merge area	M <sub>S</sub>	0.315						
Space Mean Speed in Ramp Influence Area (450m)	S <sub>R</sub>	96.4	km/h	88%	of FFS			
Number of out side lane	No	1.0						
Ave. per-lane flow rate in outer lanes			<500pc/h	> 500 -2200ma/h	>2300pc/b			
	V <sub>OA</sub>		•	>500<2300pc/h				
Space Mean Speed in outer lanes	So	101.8	110	101.8	103.37			
Conversion to pc/h under base conditions							No.of	lane
pc/h	v	PHF	%HV	f <sub>HV</sub>	fp	v		
V <sub>F</sub>	3955	0.95	12.31		-	4676		3
V <sub>R</sub>	118	0.95	3.39	0.967	1	128		1
V <sub>12</sub>						2765		
V <sub>R12</sub>						2893		
V <sub>FO</sub>						4804		3
Merge						,004		
Density of merge influence area	D <sub>R</sub>	15.34	pc/km/ln					
Level of Service	- K	10.04 C	-					
		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>				
Capacity Check		* F	• K	* FO				
Density as per Graph (Exhibit 11.6) of HCM								
2010 - pc/ln/ speed	Density	14.17	2.14					
Level of Service		C						
V/C		0.66						
FW/Ramp max. lane capacity based on speed		2350	2000	2350				

Freeway Off-ramp Diverge LOS Calcul	ation in a	ccordance	with Austro	ads/ HCM				
						•	М	
Project:	Mooney N	lonney/ Pac	ific Highway				MOTT	ALD
Job No.:	397610							
Section Description:		ghway (M1)	On-Ramp (S	outhbound)		PM 2018	6	
User:	J. Muller	5 , , ,	<u> </u>	,				
Date:	28/09/2020	)						
V <sub>R</sub>				V <sub>R</sub>	74	veh/h		
				V <sub>F</sub>	2205	veh/h		
V <sub>F</sub>								
					LV	HV	%HV	
				V <sub>R</sub>	67	7		9.46
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>	1,923	282	1	2.79
FFS - Freeway	S <sub>FF</sub>	110	km/h					
No. of lanes on freeway segment		3						
No. of lanes on On-Ramp		1						
Acceleration lane length	L <sub>A</sub>	100	m					
Distance to adjacent downstream Ramp	L <sub>dow n</sub>	N/A	m	applicable for	Equation 3	only		
Distance to adjacent upstream Ramp	L <sub>up</sub>	N/A	m	applicable for	Equation 2	only		
Passenger car equivalent for truck (HV factor)	Et	2	Given by SWTC	C. Cross check wi	ith Exhibit 23-	9, if grade is	know n.	
Proportion of approaching freeway flow								
remaining in Lanes 1 and 2 immediately upstream of merge	-		6 Lane FW	0 5007				
Intermediate speed determination variable for	P <sub>FM</sub>	0.587	o Lane Fvv	0.5867	Equation 1			
merge area	Ms	0.299						
Space Mean Speed in Ramp Influence		0.200						
Area (450m)	S <sub>R</sub>	97.2	km/h	88%	of FFS			
Number of out side lane	No	1.0						
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>	1082.0	<500pc/h	>500<2300pc/h	>2300pc/h	n		
Space Mean Speed in outer lanes	So	106.6	110	106.6	111.6602			
Conversion to pc/h under base conditions pc/h	v	PHF	%HV	£	fm	v	No.of L	ane
V <sub>F</sub>	2205			<b>f<sub>HV</sub></b> 0.887	fp 1			2
								3
V <sub>R</sub>	74	0.95	9.46	0.914	1			1
V <sub>12</sub>						1536		
V <sub>R12</sub>						1621		
V <sub>FO</sub>						2703		3
Merge	<b>D</b>	0.00	<i>"</i>					
Density of merge influence area Level of Service	D <sub>R</sub>		pc/km/ln					
		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>				
Capacity Check		۷F	۳R	♥ FO				
Density as per Graph (Exhibit 11.6) of HCM 2010 - pc/ln/ speed	Density	7.93	1.42					
Level of Service		В	А	В				
V/C		0.37						
FW/Ramp max. lane capacity based on speed		2350	2000	2350				

							М	
Project:		Ionney/ Pac	ific Highway	5			MACDONAL	
Job No.: Section Description:	397610	abwey (M4)	Off Damm /N	arth haund)	Sund	av naak ha		
User:	J. Muller	griway (wri)	Off-Ramp (N	oranbound)	Sunda	ay peak ho	Jul 2016	
Date:	28/09/2020	)						
			V <sub>R</sub>	V <sub>R</sub>	224	veh/h		
V <sub>F</sub>				V <sub>F</sub>	3,501	veh/h		
					LV	HV	%HV	
				V <sub>R</sub>	LV 224		%HV 0.0	
	0							
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>	3,362	139	3.9	
FFS - Freeway	S <sub>FF</sub>	110	km/h					
No. of lanes on freeway segment No. of lanes on Off-Ramp		3						
	1							
Deceleration lane length	L <sub>D</sub>	60	m					
Distance to adjacent downstream Ramp	L <sub>dow n</sub>	N/A	m	applicable for	Equation 7	only		
Distance to adjacent upstream Ramp	L <sub>up</sub>	N/A	m	applicable for	Equation 6	only		
Passenger car equivalent for truck (HV factor)	_	_						
Proportion of through freeway flow remaining in	Et	2	Given by SWT	C. Cross check	with Exhibi	t 23-9, if grad	de is known	
Lanes 1 and 2 immediately upstream of								
diverge	P <sub>FD</sub>	0.578	6 Lane FW	0.578	Equation 5 from Exhibit 25-12			
Intermediate speed determination variable for	_							
diverge area	D <sub>S</sub>	0.424						
Space Mean Speed in Ramp Influence Area (450m)	S <sub>R</sub>	91.8	km/h	83%	of FFS			
Number of out side lane	No	1.0						
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>	1518.0	<1000pc/h	>1000pc/h				
Space Mean Speed in outer lanes	S <sub>0</sub>	113.4	116.6	113.4				
	-0							
Conversion to pc/h under base conditions							No.of Lane	
pc/h	v	PHF	%HV	f <sub>HV</sub>	fp	v (pc/h)	No.of Lane	
-	<b>V</b> 3501	PHF 0.95			fp 1		No.of Lane	
pc/h	-	0.95	3.97	0.962	-	3832		
pc/h V <sub>F</sub> V <sub>R</sub>	3501	0.95	3.97	0.962	1	3832		
pc/h V <sub>F</sub>	3501	0.95	3.97	0.962	1	3832 236		
pc/h V <sub>F</sub> V <sub>R</sub> V <sub>12</sub> V <sub>FO</sub>	3501	0.95	3.97	0.962	1	3832 236 2314		
pc/h V <sub>F</sub> V <sub>R</sub> V <sub>12</sub> V <sub>FO</sub>	3501	0.95	3.97 0.00	0.962	1	3832 236 2314		
pc/h V <sub>F</sub> V <sub>R</sub> V <sub>12</sub> V <sub>FO</sub> Diverge Density of diverge influence area	3501	0.95	3.97 0.00 pc/km/ln	0.962	1	3832 236 2314		
<b>pc/h</b> V <sub>F</sub> V <sub>R</sub> V <sub>12</sub> V <sub>FO</sub>	3501	0.95	3.97 0.00 pc/km/ln	0.962	1	3832 236 2314		
pc/h V <sub>F</sub> V <sub>R</sub> V <sub>12</sub> V <sub>FO</sub> Diverge Density of diverge influence area Level of Service Capacity Check	3501	0.95 0.95 13.8 C	3.97 0.00 pc/km/ln	0.962	1	3832 236 2314		
pc/h V <sub>F</sub> V <sub>R</sub> V <sub>12</sub> V <sub>FO</sub> Diverge Density of diverge influence area Level of Service Capacity Check Density as per Graph (Exhibit 11.6) of HCM	3501 224 D <sub>R</sub>	0.95 0.95 13.8 C	3.97 0.00 pc/km/ln V <sub>R</sub>	0.962 1.000		3832 236 2314		
pc/h V <sub>F</sub> V <sub>R</sub> V <sub>12</sub> V <sub>FO</sub> Diverge Density of diverge influence area Level of Service Capacity Check Density as per Graph (Exhibit 11.6) of HCM 2010 - pc/ln/ speed	3501	0.95 0.95 13.8 <b>C</b> V <sub>F</sub>	3.97 0.00 pc/km/ln V <sub>R</sub> <b>3.9</b>	0.962 1.000 V <sub>FO</sub> 10.9	1 1 pc/km/ln	3832 236 2314		
pc/h V <sub>F</sub> V <sub>R</sub> V <sub>12</sub> V <sub>FO</sub> Diverge Density of diverge influence area Level of Service Capacity Check	3501 224 D <sub>R</sub>	0.95 0.95 13.8 C	3.97 0.00 pc/km/ln V <sub>R</sub> 3.9	0.962 1.000 V <sub>FO</sub> 10.9 B	pc/km/ln	3832 236 2314		

Project:	Mooney N	lonney/ Pac	ific Highway				MACDONALD	
Job No.:	397610							
Section Description:	Pacific Highway (M1) Off-Ramp (Southbound) Sunday peak hour J. Muller							
User: Date:	28/09/2020	<u>,                                     </u>						
	20/09/2020	, 						
		×	V	V	156	veh/h		
			V <sub>R</sub>	V <sub>R</sub>				
V <sub>F</sub>		•		V <sub>F</sub>	3,569	veh/h		
					LV	HV	%HV	
				V <sub>R</sub>				
	0							
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>	3,384	185	5.1	
FFS - Freeway	S <sub>FF</sub>	110	km/h					
No. of lanes on freeway segment		3						
No. of lanes on Off-Ramp		1						
Deceleration lane length	L <sub>D</sub>	60	m					
Distance to adjacent downstream Ramp	L <sub>dow n</sub>	N/A	m	applicable for	Equation 7	only		
Distance to adjacent upstream Ramp					•	•		
	L <sub>up</sub>	N/A	m	applicable for	Equation 6	only		
	_							
Passenger car equivalent for truck (HV factor)	Et	2	Given by SWT	C. Cross check	with Exhibi	t 23-9, if gra	de is known	
Proportion of through freeway flow remaining in Lanes 1 and 2 immediately upstream of								
diverge	P <sub>FD</sub>	0.574	6 Lane FW	0.574	Equation 5	from Exhil	nit 25.12	
Intermediate speed determination variable for	I FD	0.374		0.374			511 25-12	
diverge area	Ds	0.419						
Space Mean Speed in Ramp Influence	-3	00						
Area (450m)	S <sub>R</sub>	92.0	km/h	84%	of FFS			
Number of out side lane	No	1.0						
Ave. per-lane flow rate in outer lanes			. 1000 //					
	V <sub>OA</sub>	1607.4	<1000pc/h	>1000pc/h				
Space Mean Speed in outer lanes	So	112.8	116.6	112.8				
Conversion to pc/h under base conditions			o/1.D./				No.of Lane	
pc/h	v	PHF	%HV	f <sub>HV</sub>	fp	v (pc/h)		
V <sub>F</sub>	3569	0.95	5.18	0.951	1	3952	;	
V <sub>R</sub>	156	0.95	10.26	0.907	1	181		
V <sub>12</sub>						2344		
V <sub>FO</sub>						3771		
Diverse								
Diverge	_							
Density of diverge influence area	D <sub>R</sub>		pc/km/ln					
Level of Service		С						
Capacity Check		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>				
Density as per Graph (Exhibit 11.6) of HCM 2010 - pc/ln/ speed	Damait				pc/km/ln			
Level of Service	Density	С	3.0 A					
V/C		0.56	0.09					
FW/Ramp max. lane capacity based on speed		2350	2000					

Freeway Off-ramp Diverge LOS Calcul	ation in a	ccordance	with Austro	ads/ HCM			
							М
Project:	Mooney N	lonney/ Pac	ific Highway				MOTT MACDONALD
Job No.:	397610						
Section Description:		ghway (M1)	On-Ramp (N	orthbound)	Sunda	ay peak ho	our 2018
User:	J. Muller	<u> </u>					
Date:	28/09/2020	)					
V <sub>R</sub>				V <sub>R</sub>	133	veh/h	
				V <sub>F</sub>	2804	veh/h	
V <sub>F</sub>							
					LV	HV	%HV
				V <sub>R</sub>	132	1	0.75
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>	2,661	143	5.10
FFS - Freeway	S <sub>FF</sub>	110	km/h				
No. of lanes on freeway segment		3					
No. of lanes on On-Ramp		1					
Acceleration lane length	L <sub>A</sub>	150	m				
Distance to adjacent downstream Ramp	L <sub>dow n</sub>	N/A	m	applicable for	Equation 3	only	
Distance to adjacent upstream Ramp		N/A	m	applicable for	- Equation 2	only	
	L <sub>up</sub>	IN/A		applicable ioi		Offiy	
	_						
Passenger car equivalent for truck (HV factor)	Et	2	Given by SWTC	C Cross check wi	ith Exhibit 23-	9, if grade is	know n.
Proportion of approaching freeway flow remaining in Lanes 1 and 2 immediately							
upstream of merge	P <sub>FM</sub>	0.591	6 Lane FW	0 5913	Equation 1		
Intermediate speed determination variable for							
merge area	Ms	0.289					
Space Mean Speed in Ramp Influence							
Area (450m)	S <sub>R</sub>	97.6	km/h	89%	of FFS		
Number of out side lane	No	1.0					
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>	1267.8	<500pc/h	>500<2300pc/h	>2300pc/h		
Space Mean Speed in outer lanes	So	105.5	110	105.5	109.8017		
Conversion to pc/h under base conditions							No.of Lane
pc/h	v	PHF	%HV	f <sub>HV</sub>	fp	v	
V <sub>F</sub>	2804	0.95	5.10	0.951	1	3102	3
V <sub>R</sub>	133	0.95	0.75	0.993	1	141	1
V <sub>12</sub>						1834	
V <sub>R12</sub>						1975	
V <sub>FO</sub>						3243	3
Merge							
Density of merge influence area	D <sub>R</sub>	10.93	pc/km/ln				
Level of Service		В					
		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>			
Capacity Check							
Density as per Graph (Exhibit 11.6) of HCM							
2010 - pc/ln/ speed	Density	9.40					
Level of Service		В	A	В			
V/C		0.44	0.07	0.46			

Freeway Off-ramp Diverge LOS Calcul	ation in a	ccordance	with Austro	ads/ HCM			
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Project:	Mooney M	lonney/ Pac	ific Highway	5			MOTT MACDONALD
Job No.:	397610						
Section Description:	Pacific Hi	ghway (M1)	On-Ramp (S	outhbound)	Sunda	ay peak ho	our 2018
User:	J. Muller						
Date:	28/09/2020	)	1				
V <sub>R</sub>				V <sub>R</sub>	126	veh/h	
				V <sub>F</sub>	3476	veh/h	
V <sub>F</sub>							
					LV	HV	%HV
				V <sub>R</sub>	126	0	0.00
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>	3,296	180	5.18
FFS - Freeway	S <sub>FF</sub>	110	km/h				
No. of lanes on freeway segment		3					
No. of lanes on On-Ramp		1					
Acceleration lane length	L <sub>A</sub>	100	m				
Distance to adjacent downstream Ramp	L <sub>dow n</sub>	N/A	m	applicable for	Equation 3	only	
Distance to adjacent upstream Ramp		N/A			•	-	
	L <sub>up</sub>	IN/A	m	applicable for	Equation 2	only	
Passenger car equivalent for truck (HV factor)	-						
Proportion of approaching freeway flow	Et	2	Given by SWTC	2. Cross check wi	ith Exhibit 23-	9, if grade is I	know n.
remaining in Lanes 1 and 2 immediately							
upstream of merge	P <sub>FM</sub>	0.587	6 Lane FW	0.5867	Equation 1		
Intermediate speed determination variable for	• FIVI	0.007					
merge area	Ms	0.307					
Space Mean Speed in Ramp Influence							
Area (450m)	S <sub>R</sub>	96.8	km/h	88%	of FFS		
Number of out side lane	No	1.0					
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>	1590.6	<500pc/h	>500<2300pc/h	>2300pc/h		
Space Mean Speed in outer lanes	So	103.7	110	103.7	106.5745		
	0						
Conversion to pc/h under base conditions							No.of Lane
pc/h	v	PHF	%HV	f <sub>HV</sub>	fp	v	
V <sub>F</sub>	3476	0.95	5.18	0.951	1	3848	3
V <sub>R</sub>	126	0.95	0.00	1.000	1	133	1
V <sub>12</sub>						2258	
V <sub>R12</sub>						2391	
V <sub>FO</sub>						3981	
VFO Merge						5501	
Density of merge influence area	D <sub>R</sub>	13.57	pc/km/ln				
Level of Service		C					
		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>			
Capacity Check				10			
Density as per Graph (Exhibit 11.6) of HCM							
2010 - pc/ln/ speed	Density	11.66	2.21				
Level of Service		В	А	С			
V/C		0.55					
FW/Ramp max. lane capacity based on speed		2350	2000	2350			

Freeway Off-ramp Diverge LOS Calcul	ation in a	ccordance	with Austro	ads/ HCM					
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Project:	Mooney N	lonney/ Pac	ific Highway	9			MOTT		
Job No.:	397610								
Section Description:		ghway (M1)	Off-Ramp (N	orthbound)	AM 203	) - w/o dev	elopment		
User:	J. Muller								
Date:	28/09/2020	)							
			v	V	75				
			V <sub>R</sub>	V <sub>R</sub>		veh/h			
V <sub>F</sub>		,		V <sub>F</sub>	1,780	veh/h			
					LV	HV	%HV		
				V <sub>R</sub>	67	8	10.67		
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>		414	23.26		
· · · · · · · · · · · · · · · · · · ·				• •	.,		20.20		
FFS - Freeway No. of lanes on freeway segment	S <sub>FF</sub>	110 3	km/h						
No. of lanes on iffeeway segment No. of lanes on Off-Ramp		3 1							
Deceleration lane length	L <sub>D</sub>	60	m						
Distance to adjacent downstream Ramp	_			appliachte fra	Equation 7	, anly			
	L <sub>dow n</sub>	N/A	m	applicable for	•	•			
Distance to adjacent upstream Ramp	L <sub>up</sub>	N/A	m	applicable for	Equation 6	only			
Passenger car equivalent for truck (HV factor)	Et	2	Given by SWI	C Cross check	with Exhibit 23-9, if grade is knowr				
Proportion of through freeway flow remaining in		-				(10 0, 11 g)u			
Lanes 1 and 2 immediately upstream of diverge	P <sub>FD</sub>	0.672	6 Lane FW	0.672	Equation 5 from Exhibit 25-12				
Intermediate speed determination variable for diverge area	Ds	0.411							
Space Mean Speed in Ramp Influence Area (450m)	S <sub>R</sub>	92.3	km/h	84%	of FFS				
Number of out side lane	No	1.0							
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>	586.6	<1000pc/h	>1000pc/h					
Space Mean Speed in outer lanes	S <sub>0</sub>	116.6	116.6	119.2					
Conversion to pc/h under base conditions							No.of Lane		
pc/h	v	PHF	%HV	f <sub>HV</sub>	fp	v (pc/h)			
V <sub>F</sub>	1780	0.95	0.00		1	1874	3		
V <sub>R</sub>	75	0.95	10.67	0.904	1	87	1		
V <sub>12</sub>						1287			
V <sub>FO</sub>						1786	3		
Diverge									
Density of diverge influence area	D <sub>R</sub>		pc/km/ln						
Level of Service		B		V					
Capacity Check		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>					
Density as per Graph (Exhibit 11.6) of HCM									
2010 - pc/ln/ speed	Density		1.5		pc/km/ln				
Level of Service		B							
<b>V/C</b> FW/Ramp max. lane capacity based on speed		0.27 2350							
a writering max, lane capacity based of speed		2000	2000	2000					

Freeway Off-ramp Diverge LOS Calcul	ation in a	ccordance	with Austro	ads/ HCM								
						M						
Project:	Mooney M	lonney/ Pac	ific Highway	6			MOTT					
Job No.:	397610											
Section Description:	Pacific Hi	ghway (M1)	Off-Ramp (S	outhbound)	AM 203	) - w/o dev	elopment					
User:	J. Muller											
Date:	28/09/2020	)										
			V <sub>R</sub>	V <sub>R</sub>	95	veh/h						
V			VR	V <sub>R</sub> V <sub>F</sub>	5,054	veh/h						
V <sub>F</sub>				VF	5,054	ven/n						
					LV	HV	%HV					
				V <sub>R</sub>	73	22	23.16					
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>	4,274	780	15.43					
FFS - Freeway	S <sub>FF</sub>	110	km/h									
No. of lanes on freeway segment	5H	3	N11//11									
No. of lanes on Off-Ramp		1										
Deceleration lane length	L <sub>D</sub>	60	m									
Distance to adjacent downstream Ramp	-	N/A	m	applicable for	Equation 7	only						
Distance to adjacent upstream Ramp	L <sub>dow n</sub>				•	•						
Distance to adjacent upstream Ramp	L <sub>up</sub>	N/A	m	applicable for	Equation 6	only						
Deserve a serve serve in the state of the st	_											
Passenger car equivalent for truck (HV factor) Proportion of through freeway flow remaining in	Et	2	Given by SWT	C. Cross check	with Exhibi	t 23-9, if gra	de is known.					
Lanes 1 and 2 immediately upstream of												
diverge	P <sub>FD</sub>	0.512	6 Lane FW	0.512	Equation 5	5 from Exhil	oit 25-12					
Intermediate speed determination variable for												
diverge area	D <sub>S</sub>	0.414										
Space Mean Speed in Ramp Influence Area (450m)	0		km/h	940/	of FFS							
Number of out side lane	S <sub>R</sub>			04 70	0175							
	N <sub>O</sub>	1.0										
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>	2535.0	<1000pc/h	>1000pc/h								
Space Mean Speed in outer lanes	So	107.1	116.6	107.1								
Conversion to pc/h under base conditions	N	DUE	0/110/	4	£		No.of Lane					
pc/h	V	PHF	%HV	f <sub>HV</sub>	fp	v (pc/h)						
V <sub>F</sub>	5054	0.95			1	5320	3					
V <sub>R</sub>	95	0.95	23.16	0.812	1	123	1					
V <sub>12</sub>						2785						
V <sub>FO</sub>						5197	3					
Diverge												
Density of diverge influence area	D <sub>R</sub>	16.3	pc/km/ln									
Level of Service	- K	C										
Capacity Check		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>								
		• ۴	- IX	· FU								
Density as per Graph (Exhibit 11.6) of HCM												
2010 - pc/ln/ speed	Density		2.1		pc/km/ln							
Level of Service V/C		<b>C</b> 0.75										
FW/Ramp max. lane capacity based on speed		2350										

Freeway Off-ramp Diverge LOS Calcu	lation in a	ccordance	with Austro	ads/ HCM			
							М
Project:	Mooney N	lonney/ Pac	ific Highway:	9			MOTT MACDONALD
Job No.:	397610						
Section Description:	Pacific Hi	ghway (M1)	On-Ramp (N	orthbound)	AM 2030	) - w/o dev	elopment
User:	J. Muller						
Date:	28/09/2020	)					
V	۲			V <sub>R</sub>		veh/h	
				V <sub>F</sub>	1675	veh/h	
V	=		•				
					LV	HV -	%HV
				V <sub>R</sub>		7	17.50
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>	1,233	442	26.39
FFS - Freeway	SFF	110	km/h				
No. of lanes on freeway segment		3					
No. of lanes on On-Ramp		1					
Acceleration lane length	L <sub>A</sub>	150	m				
Distance to adjacent downstream Ramp	L <sub>dow n</sub>	N/A	m	applicable for	Equation 3	only	
Distance to adjacent upstream Ramp		N/A	m	applicable for	•	•	
	L <sub>up</sub>	IN/A		applicable ioi		Only	
Passenger car equivalent for truck (HV factor)	Et	2	Given by SWTC	2. Cross check w	ith Exhibit 23-	9, if grade is	know n.
Proportion of approaching freeway flow remaining in Lanes 1 and 2 immediately							
upstream of merge	P <sub>FM</sub>	0.591	6 Lane FW	0 5913	Equation 1	I	
Intermediate speed determination variable for	' FM	0.001		0.0010	Equation		
merge area	Ms	0.286					
Space Mean Speed in Ramp Influence	0						
Area (450m)	S <sub>R</sub>	97.7	km/h	89%	of FFS		
Number of out side lane	No	1.0					
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>	910.8	<500pc/h	>500<2300pc/h	>2300pc/h		
Space Mean Speed in outer lanes			•				
	So	107.6	110	107.6	113.3724		
Conversion to pc/h under base conditions							No.of Lane
pc/h	v	PHF	%HV	f <sub>HV</sub>	fp	v	
V <sub>F</sub>	1675	0.95	26.39		1	2228	3
V <sub>R</sub>	40						
	40	0.95	17.50	0.051	1		
V <sub>12</sub>						1318	
V <sub>R12</sub>						1367	
V <sub>FO</sub>						2278	3
Merge	-						
Density of merge influence area	D <sub>R</sub>		pc/km/ln				
Level of Service		B					
		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>			
Capacity Check							
Density as per Graph (Exhibit 11.6) of HCM							
2010 - pc/ln/ speed	Density	6.75					
Level of Service V/C		<b>B</b> 0.32					
FW/Ramp max. lane capacity based on spee	4	2350					

Freeway Off-ramp Diverge LOS Calcu	lation in a	ccordance	with Austro	ads/ HCM			
							М
Project:	Mooney N	lonney/ Pac	ific Highway	3			MOTT
Job No.:	397610						
Section Description:	Pacific Hi	ghway (M1)	On-Ramp (S	outhbound)	AM 2030	) - w/o dev	elopment
User:	J. Muller						
Date:	28/09/2020	)					
V				v	94	veh/h	
V	R			V <sub>R</sub>			
				V <sub>F</sub>	4581	veh/h	
V	F				LV	HV	%HV
				V <sub>R</sub>			1.00
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>R</sub>		188	
				۷F	4,595	100	4.10
FFS - Freeway	S <sub>FF</sub>	110	km/h				
No. of lanes on freeway segment No. of lanes on On-Ramp		3 1					
Acceleration lane length	L <sub>A</sub>	100	m				
Distance to adjacent downstream Ramp				and the state of	<b>F</b>	h	
	L <sub>dow n</sub>	N/A	m	applicable for	Equation 3	only	
Distance to adjacent upstream Ramp	L <sub>up</sub>	N/A	m	applicable for			
Passenger car equivalent for truck (HV factor)	Et	2	Given by SWIC	C Cross check wi	ith Exhibit 23-	9 if grade is l	know n
Proportion of approaching freeway flow						-,	
remaining in Lanes 1 and 2 immediately							
upstream of merge	P <sub>FM</sub>	0.587	6 Lane FW	0.5867	Equation 1		
Intermediate speed determination variable for merge area	Ms	0.340					
Space Mean Speed in Ramp Influence	IVIS	0.540					
Area (450m)	S <sub>R</sub>	95.4	km/h	87%	of FFS		
Number of out side lane	No	1.0					
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>	2074.8	<500pc/h	>500<2300pc/h	>2300pc/h		
Space Mean Speed in outer lanes	So	100.9	. 110	100.9	101.7323		
Conversion to pc/h under base conditions			0/1 D /				No.of Lane
pc/h	V	PHF	%HV	f <sub>HV</sub>	fp	v	
V <sub>F</sub>	4581	0.95	4.10	0.961	1	5020	:
V <sub>R</sub>	94	0.95	1.06	0.989	1	100	
V <sub>12</sub>						2945	
V <sub>R12</sub>						3045	
V <sub>FO</sub>						5120	:
Merge							
Density of merge influence area	D <sub>R</sub>		pc/km/ln				
Level of Service		C					
		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>			
Capacity Check							
Density as per Graph (Exhibit 11.6) of HCM 2010 - pc/ln/ speed	Density	15.21	1.67				
Level of Service		C					
V/C FW/Ramp max. lane capacity based on spee		0.71 2350					

Freeway Off-ramp Diverge LOS Calcul	ation in a	ccordance	with Austro	ads/ HCM					
							M		
Project:	Mooney N	lonney/ Pac	ific Highway	đ			MOTT		
Job No.:	397610								
Section Description:	Pacific Highway (M1) Off-Ramp (Northbound) PM 2030 - w/o develop								
User:	J. Muller								
Date:	28/09/2020	)							
	-		V <sub>R</sub>	V <sub>R</sub>	114	veh/h			
V <sub>F</sub>			VR	V <sub>R</sub> V <sub>F</sub>		veh/h			
¥F				۷F	5,021	ven/m			
					LV	HV	%HV		
				V <sub>R</sub>	112	2	1.75		
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>	4,515	506	10.08		
FFS - Freeway	S <sub>FF</sub>	110	km/h						
No. of lanes on freeway segment	IF	3							
No. of lanes on Off-Ramp		1							
Deceleration lane length	L <sub>D</sub>	60	m						
Distance to adjacent downstream Ramp	L <sub>dow n</sub>	N/A	m	applicable for	Equation 7	only			
Distance to adjacent upstream Ramp	L <sub>up</sub>	N/A	m	applicable for	Equation 6	only			
	∟up			applicable loi	Equation o	, only			
Passenger car equivalent for truck (HV factor)	Et	2	Given by SW/T	C. Cross check	with Exhibi	t23_9 ifara	de is known		
Proportion of through freeway flow remaining in		2	Chieff by OVVI	0.01033 0100		t 20-0, il gla	ac is known.		
Lanes 1 and 2 immediately upstream of									
diverge	P <sub>FD</sub>	0.514	6 Lane FW	0.514	Equation 5	5 from Exhib	oit 25-12		
Intermediate speed determination variable for diverge area	D	0.414							
Space Mean Speed in Ramp Influence	D <sub>S</sub>	0.414							
Area (450m)	S <sub>R</sub>	92.2	km/h	84%	of FFS				
Number of out side lane	No	1.0							
Ave. per-lane flow rate in outer lanes	-			> 1000m = //-					
•	V <sub>OA</sub>		<1000pc/h	>1000pc/h					
Space Mean Speed in outer lanes	So	107.2	116.6	107.2					
Conversion to pc/h under base conditions							No.of Lane		
pc/h	v	PHF	%HV	f <sub>HV</sub>	fp	v (pc/h)	NO.01 Lune		
V <sub>F</sub>	5021	0.95	0.00		-		3		
V <sub>R</sub>	114	0.95	1.75	0.983	1		1		
V <sub>12</sub>		0.00		0.000		2775			
V <sub>FO</sub>						5163	3		
Divorgo									
Diverge		40.0							
Density of diverge influence area Level of Service	D <sub>R</sub>	16.3 C	pc/km/ln						
Capacity Check		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>					
supusity one on		<b>▼</b> ⊢	I.K	▼ FO					
Density as per Graph (Exhibit 11.6) of HCM									
2010 - pc/ln/ speed	Density		2.0		pc/km/ln				
Level of Service V/C		0.75							
FW/Ramp max. lane capacity based on speed		2350							
,									

Freeway Off-ramp Diverge LOS Calcul	ation in a	ccordance	with Austro	ads/ HCM				_
							M	
Project:	Mooney N	lonney/ Pac	ific Highway	9			MOTT	
Job No.:	397610							
Section Description:		ghway (M1)	Off-Ramp (S	outhbound)	PM 2030	) - w/o dev	elopmen	ıt
User:	J. Muller	<u>,                                     </u>						
Date:	28/09/2020	)						
			V <sub>R</sub>	V <sub>R</sub>	51	veh/h		
V <sub>F</sub>			¥R	V <sub>R</sub>		veh/h		
• • • • • • • • • • • • • • • • • • •				۷F	2,303	ven/m		
					LV	HV	%HV	
				V <sub>R</sub>	49	2	3.	.9
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>	2,179	324	12	.9
FFS - Freeway	S <sub>FF</sub>	110	km/h					
No. of lanes on freeway segment	₹FF	3						
No. of lanes on Off-Ramp		1						
Deceleration lane length	L <sub>D</sub>	60	m					
Distance to adjacent downstream Ramp	L <sub>dow n</sub>	N/A	m	applicable for	Equation 7	only		
Distance to adjacent upstream Ramp					•	•		
	L <sub>up</sub>	N/A	m	applicable for	Equation 6	only		
Passenger car equivalent for truck (HV factor)	-	~			=			
Proportion of through freeway flow remaining in	Et	2	Given by SWI	C. Cross check	with Exhibi	t 23-9, if gra	de is know	/n.
Lanes 1 and 2 immediately upstream of								
diverge	P <sub>FD</sub>	0.637	6 Lane FW	0.637	Equation 5	from Exhib	oit 25-12	
Intermediate speed determination variable for								
diverge area	Ds	0.408						
Space Mean Speed in Ramp Influence Area (450m)	S <sub>R</sub>	92.5	km/h	84%	of FFS			
Number of out side lane	No	1.0						
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>	935 1	<1000pc/h	>1000pc/h				
•			•					
Space Mean Speed in outer lanes	So	116.6	116.6	117.0				
Conversion to pc/h under base conditions							No.of Lar	ne
pc/h	v	PHF	%HV	f <sub>HV</sub>	fp	v (pc/h)		
V <sub>F</sub>	2503	0.95	0.00		. 1			:
V <sub>R</sub>	51	0.95	3.92	0.962	1	56		
V <sub>12</sub>		0.00	0.02	0.002	· ·	1700		
						2579		
V <sub>FO</sub>						2579		
Diverge								
Density of diverge influence area	D <sub>R</sub>	10.6	pc/km/ln					
Level of Service	IX.	B	-					
Capacity Check		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>				
Density as per Graph (Exhibit 11.6) of HCM								
2010 - pc/ln/ speed	Density		0.9	7.8	pc/km/ln			
Level of Service	·····,	В	А	В				
V/C		0.37						
FW/Ramp max. lane capacity based on speed		2350	2000	2350				

Freeway Off-ramp Diverge LOS Calcul	ation in a	ccordance	with Austro	ads/ HCM					
							M		
Project:	Mooney N	lonney/ Pac	ific Highway	6			MOTT		
Job No.:	397610								
Section Description:		ghway (M1)	On-Ramp (N	orthbound)	PM 2030	) - w/o dev	elopment		
User: Date:	J. Muller 28/09/2020	<b>.</b>							
	20/03/2020								
V <sub>R</sub>				V <sub>R</sub>	134	veh/h			
				V <sub>F</sub>		veh/h			
V <sub>F</sub>			•						
					LV	HV	%HV		
				V <sub>R</sub>	129	5	3.73		
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>	3,930	551	12.30		
FFS - Freeway	S <sub>FF</sub>	110	km/h						
No. of lanes on freeway segment		3							
No. of lanes on On-Ramp		1							
Acceleration lane length	L <sub>A</sub>	150	m						
Distance to adjacent downstream Ramp	L <sub>dow n</sub>	N/A	m	applicable for	Equation 3	only			
Distance to adjacent upstream Ramp	L <sub>up</sub>	N/A	m	applicable for	Equation 2				
	чр				•	•			
Passenger car equivalent for truck (HV factor)	E,	2	Civon by SMTC	Cross shock wi	ith Evhibit 23	0 if grado is l	(DOW D		
Proportion of approaching freeway flow	L-t	2	Given by Sviic		IUT EXTIIDIL 23-	th Exhibit 23-9, if grade is k			
remaining in Lanes 1 and 2 immediately									
upstream of merge	P <sub>FM</sub>	0.591	6 Lane FW	0.5913	Equation 1				
Intermediate speed determination variable for merge area	5.4	0.250							
Space Mean Speed in Ramp Influence	M <sub>S</sub>	0.359							
Area (450m)	S <sub>R</sub>	94.6	km/h	86%	of FFS				
Number of out side lane	No	1.0							
Ave. per-lane flow rate in outer lanes			<500pc/h	500.0000 //	> 2200ma/h				
•	V <sub>OA</sub>			>500<2300pc/h					
Space Mean Speed in outer lanes	So	100.3	110	100.3	100.8318				
Conversion to pc/h under base conditions							No.of Lane		
pc/h	v	PHF	%HV	f <sub>HV</sub>	fp	v			
V <sub>F</sub>	4481	0.95	12.30	0.891	1	5297	3		
V <sub>R</sub>	134								
	134	0.95	3.73	0.904	1				
V <sub>12</sub>						3132			
V <sub>R12</sub>						3278			
V <sub>FO</sub>						5443	3		
Merge									
Density of merge influence area	D <sub>R</sub>	17.19	pc/km/ln						
Level of Service		D							
		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>					
Capacity Check									
Density as per Graph (Exhibit 11.6) of HCM									
2010 - pc/ln/ speed	Density	16.05							
Level of Service V/C		0.75							
FW/Ramp max. lane capacity based on speed		2350							

Freeway Off-ramp Diverge LOS Calcu	ation in a	ccordance	with Austro	ads/ HCM			
							М
Project:	Mooney N	lonney/ Pac	ific Highway:	8			
Job No.:	397610						
Section Description:	Pacific Hi	ghway (M1)	On-Ramp (S	outhbound)	PM 2030	) - w/o dev	elopment
User:	J. Muller						
Date:	28/09/2020	)			1		
					-		
V <sub>F</sub>				V <sub>R</sub>		veh/h	
				V <sub>F</sub>	2498	veh/h	
V <sub>i</sub>	-		•				
					LV	HV	%HV
				V <sub>R</sub>	70	8	10.2
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>	2,179	319	12.7
FFS - Freeway	S <sub>FF</sub>	110	km/h				
No. of lanes on freeway segment		3					
No. of lanes on On-Ramp		1					
Acceleration lane length	L <sub>A</sub>	100	m				
Distance to adjacent downstream Ramp	L <sub>dow n</sub>	N/A	m	applicable for	Equation 3	only	
Distance to adjacent upstream Ramp		N/A	m	applicable for	-		
	L <sub>up</sub>			applicable ioi			
Deserves and the least for twelv (11) ( footer)	-	_					
Passenger car equivalent for truck (HV factor) Proportion of approaching freeway flow	Et	2	Given by SWTC	C. Cross check wi	ith Exhibit 23-	know n.	
remaining in Lanes 1 and 2 immediately							
upstream of merge	P <sub>FM</sub>	0.587	6 Lane FW	0.5867	Equation 1		
Intermediate speed determination variable for					•		
merge area	M <sub>S</sub>	0.300					
Space Mean Speed in Ramp Influence Area (450m)	c	07.1	km/h	000/	of FFS		
	S <sub>R</sub>			00 70	01773		
Number of out side lane	N <sub>O</sub>	1.0					
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>	1225.5	<500pc/h	>500<2300pc/h	>2300pc/h		
Space Mean Speed in outer lanes	So	105.8	110	105.8	110.2246		
Conversion to pc/h under base conditions							No.of Lane
pc/h	v	PHF	%HV	f <sub>HV</sub>	fp	v	
V <sub>F</sub>	2498	0.95	12.77		-	2965	
V <sub>R</sub>	78	0.95			1	91	
V <sub>12</sub>						1740	
V <sub>R12</sub>						1830	
V <sub>FO</sub>						3056	
Merge						0000	
Density of merge influence area	D <sub>R</sub>	10.89	pc/km/ln				
Level of Service		В					
		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>			
Capacity Check							
Density as per Graph (Exhibit 11.6) of HCM							
2010 - pc/ln/ speed	Density	8.99					
Level of Service		B					
V/C FW/Ramp max. lane capacity based on speed		0.42 2350					

							М	
Project:	Mooney N	/onney/ Pac	ific Highway	,				
Job No.:	397610	-						
Section Description:	Pacific Hi	ghway (M1)	Off-Ramp (N	orthbound)	Sunday	PH 2030 - w/	o de ve lopme n	t
User:	J. Muller	51						
Date:	28/09/2020	)						
					05.4	1.0		
			V <sub>R</sub>	V <sub>R</sub>		veh/h		
V <sub>F</sub>		,		V <sub>F</sub>	3,967	veh/h		
					LV	HV	%HV	
				V <sub>R</sub>				0.0
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>		157	9	3.9
· ·				•F	0,010			
FFS - Freeway	S <sub>FF</sub>	110	km/h					
No. of lanes on freeway segment No. of lanes on Off-Ramp		3 1						
Deceleration lane length		60	m					
•	L <sub>D</sub>							
Distance to adjacent downstream Ramp	L <sub>dow n</sub>	N/A	m	applicable for	Equation 7	only		
Distance to adjacent upstream Ramp	L <sub>up</sub>	N/A	m	applicable for	Equation 6	only		
Passenger car equivalent for truck (HV factor)	Et	2	Given by SWT	C. Cross check	with Exhibi	t 23-9, if gra	de is known.	
Proportion of through freeway flow remaining in Lanes 1 and 2 immediately upstream of								
diverge	D	0.561	6 Lane FW	0.561	Equation 6	5 from Exhil	ait 25 12	
Intermediate speed determination variable for	P <sub>FD</sub>	0.301		0.001			JIL 2J-12	
diverge area	Ds	0.427						
Space Mean Speed in Ramp Influence								
Area (450m)	S <sub>R</sub>	91.6	km/h	83%	of FFS			
Number of out side lane	No	1.0						
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>	1714.9	<1000pc/h	>1000pc/h				
Space Mean Speed in outer lanes		112.2	116.6					
	So	112.2	110.0	112.2				
Conversion to pc/h under base conditions							No.of Lane	
pc/h	v	PHF	%HV	f <sub>HV</sub>	fp	v (pc/h)		
V <sub>F</sub>	3967	0.95	0.00	1.000	1	4176		
V <sub>R</sub>	254	0.95	0.00	1.000	1	267		
	204	0.00	0.00	1.000				
V <sub>12</sub>						2461		
V <sub>FO</sub>						3908		
Diverge								
Density of diverge influence area	D <sub>R</sub>		pc/km/ln					
Level of Service		C						
Capacity Check		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>				
Density as per Granh (Exhibit 11.6) of LONA								
Density as per Graph (Exhibit 11.6) of HCM 2010 - pc/ln/ speed	Density		4.5	11.8	pc/km/ln			
Level of Service	Senory	С						
V/C		0.59						
FW/Ramp max. lane capacity based on speed		2350	2000	2350				

							М	м
Project:	Mooney M	lonney/ Pac	ific Highway				MOTT	
Job No.:	397610							
Section Description:		ghway (M1)	Off-Ramp (Se	outhbound)	Sunday P	H 2030 - w/o	de ve lopn	nent
User: Date:	J. Muller 28/09/2020	v						
Date.	28/09/2020	)						
			V <sub>R</sub>	V <sub>R</sub>	175	veh/h		
V <sub>F</sub>			¥R	V <sub>R</sub> V <sub>F</sub>		veh/h		
۷F				۷F	4,045	ven/m		
					LV	HV	%HV	
				V <sub>R</sub>	159	16		9.1
FFS -Ramp	S <sub>FR</sub>	60	km/h	VF	3,836	209		5.1
FFS - Freeway	S <sub>FF</sub>	110	km/h					
No. of lanes on freeway segment	<b>~</b> FF	3						
No. of lanes on Off-Ramp		1						
Deceleration lane length	L <sub>D</sub>	60	m					
Distance to adjacent downstream Ramp	L <sub>dow n</sub>	N/A	m	applicable for	Equation 7	only		
Distance to adjacent upstream Ramp					-	-		
	L <sub>up</sub>	N/A	m	applicable for	Equation 6	only		
Passenger car equivalent for truck (HV factor)	-							
Proportion of through freeway flow remaining in	Et	2	Given by SWT	C. Cross check	with Exhibi	t 23-9, if gra	de is knov	wn.
Lanes 1 and 2 immediately upstream of								
diverge	P <sub>FD</sub>	0.559	6 Lane FW	0.559	Equation 5	from Exhil	oit 25-12	
Intermediate speed determination variable for diverge area	Ds	0.421						
Space Mean Speed in Ramp Influence								
Area (450m)	S <sub>R</sub>	91.9	km/h	84%	of FFS			
Number of out side lane	No	1.0						
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>	1788.6	<1000pc/h	>1000pc/h				
Space Mean Speed in outer lanes	S <sub>O</sub>	111.7	116.6					
	00	111.7	110.0	111.7				
Conversion to pc/h under base conditions							No.of La	ne
pc/h	v	PHF	%HV	f <sub>HV</sub>	fp	v (pc/h)		
V <sub>F</sub>	4045	0.95	0.00	1.000	1	4258		;
V <sub>R</sub>	175	0.95	9.14	0.916	1	201		
V <sub>12</sub>						2469		
V <sub>FO</sub>						4057		
Diverse								
Diverge	<b>D</b>							
Density of diverge influence area	D <sub>R</sub>		pc/km/ln					
Level of Service Capacity Check		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>				
Density as per Graph (Exhibit 11.6) of HCM 2010 - pc/ln/ speed	Darreit			40.0	Do///			
	Density	С	3.4 A	1	pc/km/ln			
Level of Service								
Level of Service V/C		0.60	0.10					

Freeway Off-ramp Diverge LOS Calcul	ation in a	ccordance	with Austro	ads/ HCM			
						•	M MOTT M
Project:	Mooney N	lonney/ Pac	ific Highway	8			MACDONALD
Job No.:	397610						
Section Description:		ghway (M1)	On-Ramp (N	orthbound)	Sunday PH	1 2030 - w/o d	levelopment
User: Date:	J. Muller 28/09/2020	<u>,</u>					
	20/00/2020						
V <sub>R</sub>				V <sub>R</sub>	150	veh/h	
				V <sub>F</sub>		veh/h	
V <sub>F</sub>			•	• •			
					LV	HV	%HV
				V <sub>R</sub>	149	1	0.6
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>	3,556	157	4.2
FFS - Freeway	S <sub>FF</sub>	110	km/h				
No. of lanes on freeway segment		3					
No. of lanes on On-Ramp		1					
Acceleration lane length	L <sub>A</sub>	150	m				
Distance to adjacent downstream Ramp	L <sub>dow n</sub>	N/A	m	applicable for	Equation 3	only	
Distance to adjacent upstream Ramp	L <sub>up</sub>	N/A	m	applicable for	Equation 2		
Passenger car equivalent for truck (HV factor)	Et	2	Given by SWT(	C. Cross check wi	ith Exhibit 23-	know n	
Proportion of approaching freeway flow		-	Given by Gvvic			o, il glade io i	
remaining in Lanes 1 and 2 immediately							
upstream of merge	P <sub>FM</sub>	0.591	6 Lane FW	0.5913	Equation 1	1	
Intermediate speed determination variable for merge area	N.4	0.299					
Space Mean Speed in Ramp Influence	M <sub>S</sub>	0.299					
Area (450m)	S <sub>R</sub>	97.1	km/h	88%	of FFS		
Number of out side lane	No	1.0					
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>	1664.9	<500pc/h	>500<2300pc/h	>2300pc/h	1	
Space Mean Speed in outer lanes		103.2			105.8309		
	So	103.2	110	103.2	105.8509		
Conversion to pc/h under base conditions							No.of Lane
pc/h	v	PHF	%HV	f <sub>HV</sub>	fp	v	
VF	3713	0.95	4.23	0.959	1	4074	:
V <sub>R</sub>	150	0.95	0.67	0.993	1	159	
V <sub>12</sub>						2409	
V <sub>R12</sub>						2568	
V <sub>FO</sub>						4233	
Merge						-200	
Density of merge influence area	D <sub>R</sub>	13.77	pc/km/ln				
Level of Service		С	-				
		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>			
Capacity Check							
Density as per Graph (Exhibit 11.6) of HCM							
2010 - pc/ln/ speed	Density	12.34					
Level of Service V/C		0.58					
FW/Ramp max. lane capacity based on speed		2350					

Freeway Off-ramp Diverge LOS Calcul	ation in a	ccordance	with Austro	ads/ HCM			
Project:	Mooney	Jonnov/ Pag	ific Highway:				M MOTT M
		ionney/ Fac	and nighway				MACDONALD
Job No.:	397610		0				
Section Description: User:	J. Muller	ghway (M1)	On-Ramp (S	outhbound)	Sunday PH	12030 - w/o c	levelopment
Date:	28/09/2020	)					
V <sub>R</sub>				V <sub>R</sub>	158	veh/h	
				V <sub>F</sub>	3870	veh/h	
V <sub>F</sub>			•				
					LV	HV	%HV
				V <sub>R</sub>	143	15	9.49
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>	3,677	193	4.99
FFS - Freeway	S <sub>FF</sub>	110	km/h				
No. of lanes on freeway segment		3					
No. of lanes on On-Ramp	1	1	m				
Acceleration lane length Distance to adjacent downstream Ramp	L <sub>A</sub>	100	m		<b>F</b>		
•	L <sub>dow n</sub>	N/A	m	applicable for	•		
Distance to adjacent upstream Ramp	L <sub>up</sub>	N/A	m	applicable for	Equation 2	only	
Decourses can involve for truck (LIV footor)	-	_					
Passenger car equivalent for truck (HV factor) Proportion of approaching freeway flow	Et	2	Given by SWTC	C Cross check wi	ith Exhibit 23-	know n.	
remaining in Lanes 1 and 2 immediately							
upstream of merge	P <sub>FM</sub>	0.587	6 Lane FW	0.5867	Equation 1	l	
Intermediate speed determination variable for							
merge area	M <sub>S</sub>	0.316					
Space Mean Speed in Ramp Influence Area (450m)	S <sub>R</sub>	96.4	km/h	88%	of FFS		
Number of out side lane	No	1.0					
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>	1767.6	<500pc/h	>500<2300pc/h	>2300pc/h	1	
Space Mean Speed in outer lanes	So	102.6	110	102.6	104.8038		
							No of Long
Conversion to pc/h under base conditions pc/h	v	PHF	%HV	f <sub>HV</sub>	fp	v	No.of Lane
V <sub>F</sub>	3870				-		
V <sub>R</sub>	158						
V <sub>12</sub>	130	0.93	3.43	0.913	1		
						2509	
V <sub>R12</sub>						2691	
V <sub>FO</sub> Merge						4459	
Density of merge influence area	D <sub>R</sub>	15.00	pc/km/ln				
Level of Service	S.K	10.00 C	•				
		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>			
Capacity Check							
Density as per Graph (Exhibit 11.6) of HCM 2010 - pc/ln/ speed	Density	12.96	3.04				
Level of Service	Density	12.96 C					
V/C		0.61		0.63			
FW/Ramp max. lane capacity based on speed		2350	2000	2350			

Freeway Off-ramp Diverge LOS Calcul		eeer aanee	740000				М			
Project:	Mooney N	lonney/ Pac	ific Highway	8			MACDONAL			
Job No.:	397610									
Section Description:	Pacific Highway (M1) Off-Ramp (Northbound) AM 2030 - with developmed J. Muller									
User: Date:	28/09/2020	<b>.</b>								
	20/00/2020									
			V <sub>R</sub>	V <sub>R</sub>	119	veh/h				
V <sub>F</sub>			- K	V <sub>F</sub>	1,824	veh/h				
					LV	ΗV	%HV			
				V <sub>R</sub>	111	8	6.7			
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>	1,410	414	22.7			
FFS - Freeway	S <sub>FF</sub>	110	km/h							
No. of lanes on freeway segment		3								
No. of lanes on Off-Ramp		1								
Deceleration lane length	L <sub>D</sub>	60	m							
Distance to adjacent downstream Ramp	L <sub>dow n</sub>	N/A	m	applicable for	Equation 7	only				
Distance to adjacent upstream Ramp		N/A	m	applicable for	•	-				
	L <sub>up</sub>		m	applicable ioi		Offiy				
Passenger car equivalent for truck (HV factor)	E	2		C. Creas shash		+ 22 0 if and	de ie kreenne			
Proportion of through freeway flow remaining in	Et	2	Given by SVVI	C. Cross check		t 23-9, ii gra	de is known			
Lanes 1 and 2 immediately upstream of										
diverge	P <sub>FD</sub>	0.668	6 Lane FW	0.668	Equation 5	from Exhib	oit 25-12			
Intermediate speed determination variable for	_									
diverge area	Ds	0.415								
Space Mean Speed in Ramp Influence Area (450m)	S <sub>R</sub>	92.2	km/h	8/0/	of FFS					
Number of out side lane				04 /0	01113					
	N <sub>O</sub>	1.0								
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>	592.5	<1000pc/h	>1000pc/h						
Space Mean Speed in outer lanes	So	116.6	116.6	119.1						
Conversion to pc/h under base conditions			o		-		No.of Lane			
pc/h	V	PHF	%HV	f <sub>HV</sub>	fp	v (pc/h)				
V <sub>F</sub>	1824	0.95	0.00	1.000	1	1920	:			
V <sub>R</sub>	119	0.95	6.72	0.937	1	134				
V <sub>12</sub>						1328				
V <sub>FO</sub>						1786				
*FU						1700				
Diverge										
Density of diverge influence area	D-	2 2	pc/km/ln							
Level of Service	D <sub>R</sub>	0.0 B	-							
Capacity Check		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>						
Capacity Check		۷F	¥R	♥ FO						
Density as per Graph (Exhibit 11.6) of HCM										
2010 - pc/ln/ speed	Density		2.2		pc/km/ln					
Level of Service		B								
<b>V/C</b> FW/Ramp max. lane capacity based on speed		0.27 2350								
withamp max, lane capacity based off speed		2000	2000	2000						

				oads/ HCM			M	
							Μ	N
Project:	Mooney N	lonney/ Pac	ific Highway	8			MACDO	ONALI
Job No.:	397610							
Section Description: User:	Pacific Hig	ghway (M1)	Off-Ramp (So	outhbound)	AM 2030	- with dev	elopm	ent
Date:	28/09/2020	)						
			V <sub>R</sub>	V <sub>R</sub>	99	veh/h		
V <sub>F</sub>				V <sub>F</sub>		veh/h		
					LV	HV	%HV	
				V <sub>R</sub>	92	7		7.0
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>	4,293	780	1	15.3
FFS - Freeway	SFF	110	km/h					
No. of lanes on freeway segment		3						
No. of lanes on Off-Ramp		1						
Deceleration lane length	L <sub>D</sub>	60	m					
Distance to adjacent downstream Ramp	L <sub>dow n</sub>	N/A	m	applicable for	Equation 7	only		
Distance to adjacent upstream Ramp	L <sub>up</sub>	N/A	m	applicable for	Equation 6	only		
- · ·	-up					<b>,</b>		
Passenger car equivalent for truck (HV factor)	Et	2	Given by SWT	C. Cross check	with Exhibi	t 23-9 if arau	de is kno	own
Proportion of through freeway flow remaining in	-t	-				. 20 0, ii giu		
Lanes 1 and 2 immediately upstream of								
diverge	P <sub>FD</sub>	0.512	6 Lane FW	0.512	Equation 5	from Exhib	oit 25-12	2
Intermediate speed determination variable for diverge area	D	0 4 4 2						
Space Mean Speed in Ramp Influence	D <sub>S</sub>	0.413						
Area (450m)	S <sub>R</sub>	92.2	km/h	84%	of FFS			
Number of out side lane		1.0		•				
	N <sub>O</sub>							
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>	2553.7	<1000pc/h	>1000pc/h				
Space Mean Speed in outer lanes	So	107.0	•	107.0				
	S <sub>O</sub>	107.0	•	107.0				
Conversion to pc/h under base conditions			116.6		fn		No.of L	ane
Conversion to pc/h under base conditions pc/h	v	PHF	116.6 %HV	f <sub>HV</sub>	fp	v (pc/h)		
Conversion to pc/h under base conditions pc/h V <sub>F</sub>	<b>V</b> 5073	PHF 0.95	116.6 %HV 0.00	<b>f<sub>HV</sub></b> 1.000	1	<b>v (pc/h)</b> 5340		
Conversion to pc/h under base conditions pc/h V <sub>F</sub> V <sub>R</sub>	v	PHF 0.95	116.6 %HV 0.00	<b>f<sub>HV</sub></b> 1.000	1	<b>v (pc/h)</b> 5340		
Conversion to pc/h under base conditions pc/h V <sub>F</sub>	<b>V</b> 5073	PHF 0.95	116.6 %HV 0.00	<b>f<sub>HV</sub></b> 1.000	1	<b>v (pc/h)</b> 5340		
Conversion to pc/h under base conditions pc/h $V_F$ $V_R$ $V_{12}$	<b>V</b> 5073	PHF 0.95	116.6 %HV 0.00	<b>f<sub>HV</sub></b> 1.000	1	v (pc/h) 5340 112		
Conversion to pc/h under base conditions pc/h V <sub>F</sub> V <sub>R</sub>	<b>V</b> 5073	PHF 0.95	116.6 %HV 0.00	<b>f<sub>HV</sub></b> 1.000	1	v (pc/h) 5340 112 2786		
Conversion to pc/h under base conditions pc/h $V_F$ $V_R$ $V_{12}$	<b>V</b> 5073	PHF 0.95	116.6 %HV 0.00	<b>f<sub>HV</sub></b> 1.000	1	v (pc/h) 5340 112 2786		
Conversion to pc/h under base conditions pc/h V <sub>F</sub> V <sub>R</sub> V <sub>12</sub> V <sub>FO</sub>	V 5073 99	PHF 0.95 0.95	116.6 %HV 0.00	<b>f<sub>HV</sub></b> 1.000	1	v (pc/h) 5340 112 2786		ane
Conversion to pc/h under base conditions pc/h V <sub>F</sub> V <sub>R</sub> V <sub>12</sub> V <sub>FO</sub>	<b>V</b> 5073	PHF 0.95 0.95	116.6 %HV 0.00 7.07	<b>f<sub>HV</sub></b> 1.000	1	v (pc/h) 5340 112 2786		
Conversion to pc/h under base conditions pc/h $V_F$ $V_R$ $V_{12}$ $V_{FO}$ Diverge Density of diverge influence area	V 5073 99	PHF 0.95 0.95 16.3 C	116.6 %HV 0.00 7.07	f <sub>HV</sub> 1.000 0.934	1	v (pc/h) 5340 112 2786		
Conversion to pc/h under base conditions pc/h $V_F$ $V_R$ $V_{12}$ $V_{FO}$ Diverge Density of diverge influence area Level of Service	V 5073 99	PHF 0.95 0.95	116.6 %HV 0.00 7.07	<b>f<sub>HV</sub></b> 1.000	1	v (pc/h) 5340 112 2786		
Conversion to pc/h under base conditions pc/h $V_F$ $V_R$ $V_{12}$ $V_{FO}$ Diverge Density of diverge influence area Level of Service Capacity Check Density as per Graph (Exhibit 11.6) of HCM	v 5073 99 D <sub>R</sub>	PHF 0.95 0.95 16.3 C	116.6 %HV 0.00 7.07 pc/km/In V <sub>R</sub>	f <sub>HV</sub> 1.000 0.934		v (pc/h) 5340 112 2786		
Conversion to pc/h under base conditions pc/h $V_F$ $V_R$ $V_{12}$ $V_{FO}$ Diverge Density of diverge influence area Level of Service Capacity Check Density as per Graph (Exhibit 11.6) of HCM 2010 - pc/ln/ speed	V 5073 99	PHF 0.95 0.95 16.3 C V <sub>F</sub>	116.6 %HV 0.00 7.07 pc/km/ln V <sub>R</sub>	f <sub>HV</sub> 1.000 0.934 V <sub>FO</sub> <b>15.8</b>	1 1 pc/km/ln	v (pc/h) 5340 112 2786		
Conversion to pc/h under base conditions pc/h $V_F$ $V_R$ $V_{12}$ $V_{FO}$ Diverge Density of diverge influence area Level of Service Capacity Check Density as per Graph (Exhibit 11.6) of HCM	v 5073 99 D <sub>R</sub>	PHF 0.95 0.95 16.3 C	116.6 %HV 0.00 7.07 pc/km/In V <sub>R</sub> 1.9	f <sub>HV</sub> 1.000 0.934 	1 1 pc/km/ln	v (pc/h) 5340 112 2786		

Freeway Off-ramp Diverge LOS Calcu	lation in a	ccordance	with Austro	ads/ HCM			
							М
Project:	Mooney N	Ionney/ Pac	ific Highway:	8			MOTT MACDONALD
Job No.:	397610						
Section Description:		ghway (M1)	On-Ramp (N	orthbound)	AM 2030	- with dev	elopment
User:	J. Muller						
Date:	28/09/2020	)					
V <sub>F</sub>	<u> </u>			V <sub>R</sub>		veh/h	
				V <sub>F</sub>	1718	veh/h	
Vi	-		•		LV	HV	%HV
				V <sub>R</sub>			8.43
	0	<u></u>					
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>	1,276	442	25.73
FFS - Freeway	S <sub>FF</sub>	110	km/h				
No. of lanes on freeway segment No. of lanes on On-Ramp		3 1					
•	1						
Acceleration lane length	L <sub>A</sub>	150	m				
Distance to adjacent downstream Ramp	L <sub>dow n</sub>	N/A	m	applicable for	Equation 3	only	
Distance to adjacent upstream Ramp	L <sub>up</sub>	N/A	m	applicable for	Equation 2		
Passenger car equivalent for truck (HV factor)	Et	2	Given by SWTC	C. Cross check wi	ith Exhibit 23-	know n.	
Proportion of approaching freeway flow	-						
remaining in Lanes 1 and 2 immediately			6 Lane FW				
upstream of merge Intermediate speed determination variable for	P <sub>FM</sub>	0.591	o Lane FVV	0.5913	Equation 1		
merge area	Ms	0.286					
Space Mean Speed in Ramp Influence		0.200					
Area (450m)	S <sub>R</sub>	97.7	km/h	89%	of FFS		
Number of out side lane	No	1.0					
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>	929.3	<500pc/h	>500<2300pc/h	>2300pc/h	h	
Space Mean Speed in outer lanes	So	107.5	110	107.5	113.1875		
	-0						
Conversion to pc/h under base conditions							No.of Lane
pc/h	v	PHF	%HV	f <sub>HV</sub>	fp	v	
V <sub>F</sub>	1718	0.95	25.73	0.795	1	2274	3
V <sub>R</sub>	83	0.95	8.43	0.922	1	95	
V <sub>12</sub>						1344	
V <sub>R12</sub>						1439	
V <sub>FO</sub>						2368	
Merge						2000	
Density of merge influence area	D <sub>R</sub>	8.37	pc/km/ln				
Level of Service	- K	B					
		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>			
Capacity Check		- F		· FU			
Density as per Graph (Exhibit 11.6) of HCM							
2010 - pc/ln/ speed	Density	6.89	1.58				
Level of Service		B					
V/C		0.32	0.05	0.34			

Freeway Off-ramp Diverge LOS Calcu	ation in a	ccordance	with Austro	ads/ HCM			
							М
Project:	Mooney N	lonney/ Pac	ific Highway	9			MOTT MACDONALD
Job No.:	397610						
Section Description:	Pacific Hi	ghway (M1)	On-Ramp (S	outhbound)	AM 2030	- with dev	elopment
User:	J. Muller						
Date:	28/09/2020	0			1		
					404	1.4	
V <sub>F</sub>				V <sub>R</sub>		veh/h	
				V <sub>F</sub>	5281	veh/h	
V <sub>f</sub>			•		LV	HV	%HV
				V <sub>R</sub>			∞⊓v 0.5
	0	00	1 1				
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>	4,493	788	14.9
FFS - Freeway	S <sub>FF</sub>	110	km/h				
No. of lanes on freeway segment		3 1					
No. of lanes on On-Ramp	1						
Acceleration lane length	L <sub>A</sub>	100	m				
Distance to adjacent downstream Ramp	L <sub>dow n</sub>	N/A	m	applicable for	Equation 3	only	
Distance to adjacent upstream Ramp	L <sub>up</sub>	N/A	m	applicable for	Equation 2		
Passenger car equivalent for truck (HV factor)	Et	2	Given by SWTC	C Cross check wi	ith Exhibit 23-	know n.	
Proportion of approaching freeway flow							
remaining in Lanes 1 and 2 immediately	_						
upstream of merge	P <sub>FM</sub>	0.587	6 Lane FW	0.5867	Equation 1		
Intermediate speed determination variable for merge area	Ms	0.647					
Space Mean Speed in Ramp Influence	IVIS	0.047					
Area (450m)	S <sub>R</sub>	82.2	km/h	75%	of FFS		
Number of out side lane	No	1.0					
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>	2640.3	<500pc/h	>500<2300pc/h	>2300pc/h		
Space Mean Speed in outer lanes	So	96.1		97.6			
	00	50.1	110	37.0	30.07000		
Conversion to pc/h under base conditions							No.of Lane
pc/h	V	PHF	%HV	f <sub>HV</sub>	fp	v	
VF	5281	0.95	14.92	0.870	1	6388	:
V <sub>R</sub>	194	0.95	0.52	0.995	1	205	
V <sub>12</sub>						3748	
V <sub>R12</sub>						3953	
V <sub>FO</sub>						6594	
Merge							
Density of merge influence area	D <sub>R</sub>	21.05	pc/km/ln				
Level of Service		D					
		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>			
Capacity Check							
Density as per Graph (Exhibit 11.6) of HCM							
2010 - pc/ln/ speed	Density	19.36					
Level of Service		D					
V/C FW/Ramp max. lane capacity based on speed		0.91 2350					

Freeway Off-ramp Diverge LOS Calcu	lation in a	ccordance	with Austro	oads/ HCM					
							м		
Project:	Mooney Monney/ Pacific Highway 397610								
Job No.:									
Section Description:		ghway (M1)	Off-Ramp (N	orthbound)	PM 2030	) - with dev	/elopment		
User:	J. Muller	<u>,                                     </u>							
Date:	28/09/2020	)							
			V <sub>R</sub>	V <sub>R</sub>	119	veh/h			
V <sub>F</sub>			VR	V <sub>R</sub> V <sub>F</sub>		veh/h			
VF				۷F	3,032	Veri/TI			
					LV	HV	%HV		
				V <sub>R</sub>	111	8	6.7		
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>	4,618	414	8.2		
FFS - Freeway	S <sub>FF</sub>	110	km/h						
No. of lanes on freeway segment		3							
No. of lanes on Off-Ramp		1							
Deceleration lane length	L <sub>D</sub>	60	m						
Distance to adjacent downstream Ramp	L <sub>dow n</sub>	N/A	m	applicable for	Equation 7	only			
Distance to adjacent upstream Ramp		N/A	m	applicable for	•	-			
	L <sub>up</sub>		10	applicable 101		July			
Passenger car equivalent for truck (HV factor)	Et	2	Given by SM/	C. Cross check	with Exhibi	+ 22 0 if are	do io known		
Proportion of through freeway flow remaining in		2	Given by Svvi	C. Closs cliect		t 23-9, ii gia			
Lanes 1 and 2 immediately upstream of									
diverge	P <sub>FD</sub>	0.513	6 Lane FW	0.513	Equation 5	5 from Exhil	oit 25-12		
Intermediate speed determination variable for diverge area	<b>_</b>	0.445							
Space Mean Speed in Ramp Influence	D <sub>S</sub>	0.415							
Area (450m)	S <sub>R</sub>	92.2	km/h	84%	of FFS				
Number of out side lane	N <sub>O</sub>	1.0							
	-								
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>	2514.4	<1000pc/h	>1000pc/h					
Space Mean Speed in outer lanes	So	107.2	116.6	107.2					
Conversion to pc/h under base conditions							No.of Lane		
pc/h	v	PHF	%HV	f <sub>HV</sub>	fp	v (pc/h)	NO.01 Lane		
V <sub>F</sub>	5032				-				
	119				1				
V <sub>R</sub>	1 19	0.95	0.72	0.937					
V <sub>12</sub>						2782			
V <sub>FO</sub>						5163			
Diverge	-								
Density of diverge influence area	D <sub>R</sub>		pc/km/ln						
Level of Service		C							
Capacity Check		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>					
Density as per Graph (Exhibit 11.6) of HCM									
2010 - pc/ln/ speed	Density		2.2	15.6	pc/km/ln				
Level of Service		С	А	C					
V/C		0.75							
FW/Ramp max. lane capacity based on speed	1	2350	2000	2350					

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Project:	Mooney Monney/ Pacific Highway Mor								
Job No.:									
Section Description:	Pacific Hi	- with dev	with development						
User:	J. Muller		• •	,					
Date:	28/09/2020	)							
					No. 2 -	1.0.00			
			V <sub>R</sub>	V <sub>R</sub>		veh/h			
V <sub>F</sub>				V <sub>F</sub>	2,503	veh/h			
					LV	HV	%HV		
				V <sub>R</sub>					
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>			12.9		
•				۷F	2,173	524	12.3		
FFS - Freeway	S <sub>FF</sub>	110	km/h						
No. of lanes on freeway segment No. of lanes on Off-Ramp		3 1							
Deceleration lane length	L <sub>D</sub>	60	m						
Distance to adjacent downstream Ramp				ann là tha th	<b>F</b>				
	L <sub>dow n</sub>	N/A	m	applicable for	-	-			
Distance to adjacent upstream Ramp	L <sub>up</sub>	N/A	m	applicable for	Equation 6	only			
Passenger car equivalent for truck (HV factor)	Et	2	Given by SWT	C. Cross check	with Exhibi	t 23-9, if gra	de is known		
Proportion of through freeway flow remaining in Lanes 1 and 2 immediately upstream of									
diverge	P <sub>FD</sub>	0.636	6 Lane FW	0.636	Equation 5	from Exhil	nit 25-12		
Intermediate speed determination variable for	• FD	0.000		0.000	Equation e				
diverge area	Ds	0.412							
Space Mean Speed in Ramp Influence									
Area (450m)	S <sub>R</sub>	92.3	km/h	84%	of FFS				
Number of out side lane	N <sub>O</sub>	1.0							
Ave. per-lane flow rate in outer lanes	VOA	921.2	<1000pc/h	>1000pc/h					
Space Mean Speed in outer lanes	So	116.6	116.6	117.1					
	-0								
Conversion to pc/h under base conditions							No.of Lane		
pc/h	v	PHF	%HV	f <sub>HV</sub>	fp	v (pc/h)			
V <sub>F</sub>	2503	0.95	0.00	1.000	1	2635			
V <sub>R</sub>	95	0.95	2.11	0.979	1	102			
V <sub>12</sub>						1713			
V <sub>FO</sub>						2533			
¥F0						2000			
Diverge									
Density of diverge influence area	D <sub>R</sub>	10.6	pc/km/ln						
Level of Service	UR	10.8 B	PORTIVIT						
Capacity Check		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>					
		۷F	۳R	♥ FO					
Density as per Graph (Exhibit 11.6) of HCM									
2010 - pc/ln/ speed	Density		1.7		pc/km/ln				
Level of Service		<b>B</b> 0.37	A 0.05						
		0.3/	0.05	0.36					
V/C FW/Ramp max. lane capacity based on speed		2350							

Freeway Off-ramp Diverge LOS Calcu	lation in a	ccordance	with Austro	ads/ HCM					
							М		
Project:	Mooney N	Mooney Monney/ Pacific Highway							
Job No.:	397610	397610							
Section Description:	Pacific Highway (M1) On-Ramp (Northbound) PM 2030 - with de								
User:	J. Muller								
Date:	28/09/2020	0	1		1				
Vi	२			V <sub>R</sub>		veh/h			
				V <sub>F</sub>	4503	veh/h			
V	F		•				0/1 B /		
					LV	HV	%HV		
				V <sub>R</sub>		5	3.2		
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>	3,952	551	12.24		
FFS - Freeway	S <sub>FF</sub>	110	km/h						
No. of lanes on freeway segment		3							
No. of lanes on On-Ramp		1							
Acceleration lane length	L <sub>A</sub>	150	m						
Distance to adjacent downstream Ramp	L <sub>dow n</sub>	N/A	m	applicable for	Equation 3	only			
Distance to adjacent upstream Ramp	L <sub>up</sub>	N/A	m	applicable for	Equation 2	only			
	-up				•	,			
Passenger car equivalent for truck (HV factor)	-	~							
Proportion of approaching freeway flow	Et	2	Given by SWIC	C. Cross check w	ith Exhibit 23-	know n.			
remaining in Lanes 1 and 2 immediately									
upstream of merge	P <sub>FM</sub>	0.591	6 Lane FW	0.5913	Equation 1				
Intermediate speed determination variable for									
merge area	M <sub>S</sub>	0.366							
Space Mean Speed in Ramp Influence	0								
Area (450m)	S <sub>R</sub>	94.3	km/h	86%	of FFS				
Number of out side lane	No	1.0							
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>	2174.3	<500pc/h	>500<2300pc/h	>2300pc/h	1			
Space Mean Speed in outer lanes	So	100.3	110	100.3	100.7372				
	-0								
Conversion to pc/h under base conditions							No.of Lane		
pc/h	V	PHF	%HV	f <sub>HV</sub>	fp	v			
V <sub>F</sub>	4503	0.95	12.24	0.891	1	5320	:		
V <sub>R</sub>	156	0.95	3.21	0.969	1	169			
V <sub>12</sub>						3146			
V <sub>R12</sub>	1					3315			
V <sub>FO</sub>	1					5489			
Merge									
Density of merge influence area	D <sub>R</sub>	17.36	pc/km/ln						
Level of Service		D	· · · · · · · · · · · · · · · · · · ·						
		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>					
Capacity Check									
Density as per Graph (Exhibit 11.6) of HCM									
2010 - pc/ln/ speed	Density	16.12	2.82						
Level of Service		C							
V/C FW/Ramp max. lane capacity based on speed		0.75 2350							

Freeway Off-ramp Diverge LOS Calcu	lation in a	ccordance	with Austro	ads/ HCM					
							М		
Project:	Mooney N	Mooney Monney/ Pacific Highway							
Job No.:	397610	397610							
Section Description:	Pacific Highway (M1) On-Ramp (Southbound) PM 2030 - with de								
User:	J. Muller								
Date:	28/09/2020	)			1				
					400	- 1.4			
Vi	<u>ا</u>			V <sub>R</sub>		veh/h			
				V <sub>F</sub>	2549	veh/h			
V	-		•		LV	HV	%HV		
				V <sub>R</sub>		8			
	•								
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>	2,230	319	12.5 <sup>-</sup>		
FFS - Freeway	S <sub>FF</sub>	110	km/h						
No. of lanes on freeway segment		3							
No. of lanes on On-Ramp		1							
Acceleration lane length	L <sub>A</sub>	100	m						
Distance to adjacent downstream Ramp	L <sub>dow n</sub>	N/A	m	applicable for	Equation 3	only			
Distance to adjacent upstream Ramp	L <sub>up</sub>	N/A	m	applicable for	Equation 2	only			
	up				•	•			
Passenger car equivalent for truck (HV factor)	E	2	o: 1 01.00						
Proportion of approaching freeway flow	Et	2	Given by Svvic	C Cross check wi	ith exhibit 23-	9, If grade is I	know n.		
remaining in Lanes 1 and 2 immediately									
upstream of merge	P <sub>FM</sub>	0.587	6 Lane FW	0.5867	Equation 1				
Intermediate speed determination variable for									
merge area	M <sub>S</sub>	0.300							
Space Mean Speed in Ramp Influence	0								
Area (450m)	S <sub>R</sub>	97.1	km/h	88%	of FFS				
Number of out side lane	No	1.0							
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>	1247.7	<500pc/h	>500<2300pc/h	>2300pc/h	1			
Space Mean Speed in outer lanes	So	105.7	110	105.7	110.0027				
Conversion to pc/h under base conditions	v	PHF	%HV	£	£		No.of Lane		
pc/h				f <sub>HV</sub>	fp	v			
VF	2549	0.95	12.51	0.889	1	3019	;		
V <sub>R</sub>	129	0.95	6.20	0.942	1	144			
V <sub>12</sub>						1771			
V <sub>R12</sub>						1915			
V <sub>FO</sub>						3163	:		
Merge									
Density of merge influence area	D <sub>R</sub>	11.28	pc/km/ln						
Level of Service		В							
		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>					
Capacity Check									
Density as per Graph (Exhibit 11.6) of HCM									
2010 - pc/ln/ speed	Density	9.15	2.40						
Level of Service	_	B							
V/C	4	0.43 2350							

							м		
Project:	Mooney Monney/ Pacific Highway								
Job No.:	397610								
Section Description:		ghway (M1)	Off-Ramp (N	orthbound)	Sunday PH	1 2030 - with	deve lopment		
User:	J. Muller	-							
Date:	28/09/2020	)							
				N/	200	e ve le /le			
	/		V <sub>R</sub>	V <sub>R</sub>		veh/h			
V <sub>F</sub>				V <sub>F</sub>	4,013	veh/h			
					LV	HV	%HV		
				V <sub>R</sub>	299	0	0.0		
FFS -Ramp	S <sub>FR</sub>	60	km/h	VF		157	3.9		
•		110	km/h	-1	· ·				
FFS - Freeway No. of lanes on freeway segment	S <sub>FF</sub>	3	NIWII						
No. of lanes on Off-Ramp		1							
Deceleration lane length	L <sub>D</sub>	60	m						
Distance to adjacent downstream Ramp		N/A	m	applicable for	Equation 7	only			
Distance to adjacent upstream Ramp	L <sub>dow n</sub>				•	•			
	L <sub>up</sub>	N/A	m	applicable for	Equation 6	only			
Passenger car equivalent for truck (HV factor)	-	_							
Proportion of through freeway flow remaining in	Et	2	Given by SWT	C. Cross check	with Exhibi	t 23-9, if gra	de is known.		
Lanes 1 and 2 immediately upstream of									
diverge	P <sub>FD</sub>	0.558	6 Lane FW	0.558	Equation 5	oit 25-12			
Intermediate speed determination variable for									
L									
diverge area	Ds	0.431							
Space Mean Speed in Ramp Influence			km/b	830/	ofEES				
Space Mean Speed in Ramp Influence Area (450m)	S <sub>R</sub>	91.5	km/h	83%	of FFS				
Space Mean Speed in Ramp Influence Area (450m) Number of out side lane	S <sub>R</sub> N <sub>O</sub>		km/h	83%	of FFS				
Space Mean Speed in Ramp Influence Area (450m)	S <sub>R</sub>	91.5 1.0	<b>km/h</b> <1000pc/h	83%	of FFS				
Space Mean Speed in Ramp Influence Area (450m) Number of out side lane	S <sub>R</sub> N <sub>O</sub>	91.5 1.0			of FFS				
Space Mean Speed in Ramp Influence Area (450m) Number of out side lane Ave. per-lane flow rate in outer lanes Space Mean Speed in outer lanes	S <sub>R</sub> N <sub>O</sub> V <sub>OA</sub>	<b>91.5</b> 1.0 1728.7	<1000pc/h	>1000pc/h	of FFS				
Space Mean Speed in Ramp Influence Area (450m) Number of out side lane Ave. per-lane flow rate in outer lanes Space Mean Speed in outer lanes Conversion to pc/h under base conditions	S <sub>R</sub> N <sub>O</sub> V <sub>OA</sub> S <sub>O</sub>	91.5 1.0 1728.7 112.1	<1000pc/h 116.6	>1000pc/h 112.1			No.of Lane		
Space Mean Speed in Ramp Influence Area (450m) Number of out side lane Ave. per-lane flow rate in outer lanes Space Mean Speed in outer lanes Conversion to pc/h under base conditions pc/h	S <sub>R</sub> N <sub>O</sub> V <sub>OA</sub> S <sub>O</sub>	91.5 1.0 1728.7 112.1 PHF	<1000pc/h 116.6 <b>%HV</b>	>1000pc/h 112.1 f <sub>HV</sub>	fp	v (pc/h)			
Space Mean Speed in Ramp Influence Area (450m) Number of out side lane Ave. per-lane flow rate in outer lanes Space Mean Speed in outer lanes Conversion to pc/h under base conditions pc/h V <sub>F</sub>	S <sub>R</sub> N <sub>O</sub> V <sub>OA</sub> S <sub>O</sub>	91.5 1.0 1728.7 112.1 PHF	<1000pc/h 116.6 %HV 0.00	>1000pc/h 112.1 <b>f<sub>HV</sub></b> 1.000	fp	v (pc/h) 4224	No.of Lane		
Space Mean Speed in Ramp Influence Area (450m) Number of out side lane Ave. per-lane flow rate in outer lanes Space Mean Speed in outer lanes Conversion to pc/h under base conditions pc/h V <sub>F</sub> V <sub>R</sub>	S <sub>R</sub> N <sub>O</sub> V <sub>OA</sub> S <sub>O</sub>	91.5 1.0 1728.7 112.1 PHF 0.95	<1000pc/h <i>116.6</i> %HV 0.00	>1000pc/h 112.1 <b>f<sub>HV</sub></b> 1.000	fp 1	4224			
Space Mean Speed in Ramp Influence Area (450m) Number of out side lane Ave. per-lane flow rate in outer lanes Space Mean Speed in outer lanes Conversion to pc/h under base conditions pc/h V <sub>F</sub> V <sub>R</sub>	S <sub>R</sub> N <sub>O</sub> V <sub>OA</sub> S <sub>O</sub> V 4013	91.5 1.0 1728.7 112.1 PHF 0.95	<1000pc/h 116.6 %HV 0.00	>1000pc/h 112.1 <b>f<sub>HV</sub></b> 1.000	fp 1	4224			
Space Mean Speed in Ramp Influence Area (450m) Number of out side lane Ave. per-lane flow rate in outer lanes Space Mean Speed in outer lanes Conversion to pc/h under base conditions pc/h V <sub>F</sub> V <sub>R</sub> V <sub>12</sub>	S <sub>R</sub> N <sub>O</sub> V <sub>OA</sub> S <sub>O</sub> V 4013	91.5 1.0 1728.7 112.1 PHF 0.95	<1000pc/h 116.6 %HV 0.00	>1000pc/h 112.1 <b>f<sub>HV</sub></b> 1.000	fp 1	4224 315			
Space Mean Speed in Ramp Influence Area (450m) Number of out side lane Ave. per-lane flow rate in outer lanes Space Mean Speed in outer lanes Conversion to pc/h under base conditions pc/h V <sub>F</sub> V <sub>R</sub>	S <sub>R</sub> N <sub>O</sub> V <sub>OA</sub> S <sub>O</sub> V 4013	91.5 1.0 1728.7 112.1 PHF 0.95	<1000pc/h 116.6 %HV 0.00	>1000pc/h 112.1 <b>f<sub>HV</sub></b> 1.000	fp 1	4224 315 2496			
Space Mean Speed in Ramp Influence Area (450m) Number of out side lane Ave. per-lane flow rate in outer lanes Space Mean Speed in outer lanes Conversion to pc/h under base conditions pc/h V <sub>F</sub> V <sub>R</sub> V <sub>12</sub>	S <sub>R</sub> N <sub>O</sub> V <sub>OA</sub> S <sub>O</sub> V 4013	91.5 1.0 1728.7 112.1 PHF 0.95	<1000pc/h 116.6 %HV 0.00	>1000pc/h 112.1 <b>f<sub>HV</sub></b> 1.000	fp 1	4224 315 2496			
Space Mean Speed in Ramp Influence Area (450m) Number of out side lane Ave. per-lane flow rate in outer lanes Space Mean Speed in outer lanes Conversion to pc/h under base conditions pc/h V <sub>F</sub> V <sub>R</sub> V <sub>R</sub> V <sub>12</sub> V <sub>FO</sub> Diverge	S <sub>R</sub> N <sub>O</sub> V <sub>OA</sub> S <sub>O</sub> V 4013 299	91.5 1.0 1728.7 112.1 PHF 0.95 0.95	<1000pc/h 116.6 %HV 0.00 0.00	>1000pc/h 112.1 <b>f<sub>HV</sub></b> 1.000	fp 1	4224 315 2496			
Space Mean Speed in Ramp Influence Area (450m) Number of out side lane Ave. per-lane flow rate in outer lanes Space Mean Speed in outer lanes Conversion to pc/h under base conditions pc/h V <sub>F</sub> V <sub>R</sub> V <sub>R</sub> V <sub>12</sub> V <sub>FO</sub>	S <sub>R</sub> N <sub>O</sub> V <sub>OA</sub> S <sub>O</sub> V 4013	91.5 1.0 1728.7 112.1 PHF 0.95 0.95	<1000pc/h 116.6 %HV 0.00 0.00	>1000pc/h 112.1 <b>f<sub>HV</sub></b> 1.000	fp 1	4224 315 2496			
Space Mean Speed in Ramp Influence Area (450m) Number of out side lane Ave. per-lane flow rate in outer lanes Space Mean Speed in outer lanes Conversion to pc/h under base conditions pc/h V <sub>F</sub> V <sub>R</sub> V <sub>R</sub> V <sub>12</sub> V <sub>FO</sub> Diverge Density of diverge influence area	S <sub>R</sub> N <sub>O</sub> V <sub>OA</sub> S <sub>O</sub> V 4013 299	91.5 1.0 1728.7 112.1 PHF 0.95 0.95 14.8	<1000pc/h 116.6 %HV 0.00 0.00	>1000pc/h 112.1 <b>f<sub>HV</sub></b> 1.000	fp 1	4224 315 2496			
Space Mean Speed in Ramp Influence Area (450m) Number of out side lane Ave. per-lane flow rate in outer lanes Space Mean Speed in outer lanes Conversion to pc/h under base conditions pc/h V <sub>F</sub> V <sub>R</sub> V <sub>R</sub> V <sub>12</sub> V <sub>FO</sub> Diverge Density of diverge influence area Level of Service Capacity Check	S <sub>R</sub> N <sub>O</sub> V <sub>OA</sub> S <sub>O</sub> V 4013 299	91.5 1.0 1728.7 112.1 PHF 0.95 0.95 14.8 C	<1000pc/h 116.6 %HV 0.00 0.00	>1000pc/h 112.1 f <sub>HV</sub> 1.000 1.000	fp 1	4224 315 2496			
Space Mean Speed in Ramp Influence Area (450m) Number of out side lane Ave. per-lane flow rate in outer lanes Space Mean Speed in outer lanes Conversion to pc/h under base conditions pc/h V <sub>F</sub> V <sub>R</sub> V <sub>R</sub> V <sub>12</sub> V <sub>FO</sub> Diverge Density of diverge influence area Level of Service Capacity Check Density as per Graph (Exhibit 11.6) of HCM	S <sub>R</sub> N <sub>O</sub> V <sub>OA</sub> S <sub>O</sub> V 4013 299 D <sub>R</sub>	91.5 1.0 1728.7 112.1 PHF 0.95 0.95 14.8 C	<1000pc/h <i>116.6</i> %HV 0.00 0.00 pc/km/ln V <sub>R</sub>	>1000pc/h 112.1 f <sub>HV</sub> 1.000 1.000	fp 1 1	4224 315 2496			
Space Mean Speed in Ramp Influence Area (450m) Number of out side lane Ave. per-lane flow rate in outer lanes Space Mean Speed in outer lanes Conversion to pc/h under base conditions pc/h V <sub>F</sub> V <sub>R</sub> V <sub>R</sub> V <sub>12</sub> V <sub>12</sub> V <sub>FO</sub> Diverge Density of diverge influence area Level of Service Capacity Check Density as per Graph (Exhibit 11.6) of HCM 2010 - pc/ln/ speed	S <sub>R</sub> N <sub>O</sub> V <sub>OA</sub> S <sub>O</sub> V 4013 299	91.5 1.0 1728.7 112.1 PHF 0.95 0.95 0.95 14.8 C V <sub>F</sub>	<1000pc/h 116.6 %HV 0.00 0.00 pc/km/ln V <sub>R</sub> 5.2	>1000pc/h 112.1 f <sub>HV</sub> 1.000 1.000 V <sub>FO</sub>	fp 1 1	4224 315 2496			
Space Mean Speed in Ramp Influence Area (450m) Number of out side lane Ave. per-lane flow rate in outer lanes Space Mean Speed in outer lanes Conversion to pc/h under base conditions pc/h V <sub>F</sub> V <sub>R</sub> V <sub>R</sub> V <sub>12</sub> V <sub>FO</sub> Diverge Density of diverge influence area Level of Service Capacity Check Density as per Graph (Exhibit 11.6) of HCM	S <sub>R</sub> N <sub>O</sub> V <sub>OA</sub> S <sub>O</sub> V 4013 299 D <sub>R</sub>	91.5 1.0 1728.7 112.1 PHF 0.95 0.95 14.8 C	<1000pc/h 116.6 %HV 0.00 0.00 pc/km/ln V <sub>R</sub> 5.2 A	>1000pc/h 112.1 f <sub>HV</sub> 1.000 1.000 V <sub>FO</sub> 11.8 B	fp 1 1	4224 315 2496			

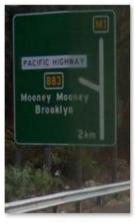
Freeway Off-ramp Diverge LOS Calcul	auon in d		Mul Musul				14			
							MOTT	м		
Project:	Mooney M	Mooney Monney/ Pacific Highway								
Job No.:	397610									
Section Description:		ghway (M1)	Off-Ramp (S	outhbound)	Sunday P	H 2030 - with	developn	nent		
User: Date:	J. Muller	<u>,</u>								
Date.	28/09/2020	8/09/2020								
			V <sub>R</sub>	V <sub>R</sub>	179	veh/h				
V	/		<b>v</b> R	V <sub>R</sub> V <sub>F</sub>	4,064	veh/h				
V <sub>F</sub>				۷F	4,004	ven/n				
					LV	HV	%HV			
				V <sub>R</sub>	178	1		0.5		
FFS -Ramp	S <sub>FR</sub>	60	km/h	VF	3,855	209		5.1		
•					-,					
FFS - Freeway No. of lanes on freeway segment	S <sub>FF</sub>	110 3	km/h							
No. of lanes on Off-Ramp		3 1								
Deceleration lane length	L <sub>D</sub>	60	m							
Distance to adjacent downstream Ramp		N/A		applicable for	Equation 7	i only				
, ,	L <sub>dow n</sub>		m		•					
Distance to adjacent upstream Ramp	L <sub>up</sub>	N/A	m	applicable for	Equation 6					
Passenger car equivalent for truck (HV factor)	Et	2	Given by SWT	C. Cross check	with Exhibi	t 23-9, if gra	de is knov	vn.		
Proportion of through freeway flow remaining in Lanes 1 and 2 immediately upstream of										
diverge	P <sub>FD</sub>	0.558	6 Lane FW	0,558	Equation 5 from Exhibit 25-12					
Intermediate speed determination variable for	· FD									
diverge area	Ds	0.420								
Space Mean Speed in Ramp Influence	-									
Area (450m)	S <sub>R</sub>	91.9	km/h	84%	of FFS					
Number of out side lane	N <sub>O</sub>	1.0								
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>	1805.1	<1000pc/h	>1000pc/h						
Space Mean Speed in outer lanes	So	111.6	116.6	111.6						
	-0									
Conversion to pc/h under base conditions							No.of La	ne		
pc/h	v	PHF	%HV	f <sub>HV</sub>	fp	v (pc/h)				
V <sub>F</sub>	4064	0.95	0.00	1.000	1	4278		:		
V <sub>R</sub>	179	0.95	0.56	0.994	1	189				
V <sub>12</sub>						2473				
V <sub>FO</sub>						4088				
Diverge										
	D	116	pc/km/ln							
Density of diverge influence area Level of Service	D <sub>R</sub>	14.0 C	pc/km/in							
			V	V						
Capacity Check		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>						
Density as per Graph (Exhibit 11.6) of HCM										
2010 - pc/ln/ speed	Density		3.2	12.4	pc/km/ln					
Level of Service		C	A							
V/C FW/Ramp max. lane capacity based on speed		0.61	0.09							
r vv/rtamp max, lane capacity pased on speed		2350	2000	2350						

Freeway Off-ramp Diverge LOS Calcul	ation in a	ccordance	with Austro	ads/ HCM				
							М	
Project:	Mooney N	Mooney Monney/ Pacific Highway						
Job No.:	397610							
Section Description:	Pacific Highway (M1) On-Ramp (Northbound) Sunday PH 2030 - with							
User:	J. Muller 28/09/2020	<u></u>						
Date:	28/09/2020	,						
V <sub>R</sub>				V <sub>R</sub>	193	veh/h		
¥R				V <sub>R</sub>		veh/h		
V <sub>F</sub>				۷F	5221	VCI//II		
••F					LV	HV	%HV	
				V <sub>R</sub>			0.5	
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>		162		
FFS - Freeway		110	km/h	- r	-,			
No. of lanes on freeway segment	S <sub>FF</sub>	3	NII/II					
No. of lanes on On-Ramp		1						
Acceleration lane length	L <sub>A</sub>	150	m					
Distance to adjacent downstream Ramp	L <sub>dow n</sub>	N/A	m	applicable for	Equation 3	only		
Distance to adjacent upstream Ramp	L <sub>up</sub>	N/A	m	applicable for	Equation 2	only		
	-up							
Passenger car equivalent for truck (HV factor)	Et	2	Given by SWTC	. Cross check wi	ith Evhibit 23	0 if grado is l	(DOW D	
Proportion of approaching freeway flow	Lt	2	Given by Sviic			9, II graue is i	NIOW II.	
remaining in Lanes 1 and 2 immediately								
upstream of merge	P <sub>FM</sub>	0.591	6 Lane FW	0.5913	Equation 1			
Intermediate speed determination variable for merge area	N.4	0.000						
Space Mean Speed in Ramp Influence	M <sub>S</sub>	0.293						
Area (450m)	S <sub>R</sub>	97.4	km/h	89%	of FFS			
Number of out side lane	No	1.0						
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>		<500pc/h	>500<2300pc/h	>2300pc/h			
Space Mean Speed in outer lanes	S <sub>O</sub>	104.5		104.5				
	30	104.5	110	104.5	101.920			
Conversion to pc/h under base conditions							No.of Lane	
pc/h	v	PHF	%HV	f <sub>HV</sub>	fp	v		
VF	3221	0.95	5.03	0.952	1	3561	:	
V <sub>R</sub>	193	0.95	0.52	0.995	1	204		
V <sub>12</sub>						2106		
V <sub>R12</sub>						2310		
V <sub>FO</sub>						3765		
Merge						5705		
Density of merge influence area	D <sub>R</sub>	12.52	pc/km/ln					
Level of Service		С						
		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>				
Capacity Check								
Density as per Graph (Exhibit 11.6) of HCM								
2010 - pc/ln/ speed Level of Service	Density	10.79						
V/C		<b>B</b> 0.51						
FW/Ramp max. lane capacity based on speed		2350						

Freeway Off-ramp Diverge LOS Calcul	ation in a	ccordance	with Austro	oads/ HCM					
Project:	Mooney N	Mooney Monney/ Pacific Highway							
Job No.:	397610	397610							
Section Description:	Pacific Highway (M1) On-Ramp (Southbound) Sunday PH 2030 - with								
User:	J. Muller								
Date:	28/09/2020	)			·				
V <sub>R</sub>				V <sub>R</sub>	244	veh/h			
				V <sub>F</sub>	4039	veh/h			
V <sub>F</sub>			•						
					LV	HV	%HV		
				V <sub>R</sub>	244	0	0.0		
FFS -Ramp	S <sub>FR</sub>	60	km/h	V <sub>F</sub>	3,836	203	5.0		
FFS - Freeway	S <sub>FF</sub>	110	km/h						
No. of lanes on freeway segment		3							
No. of lanes on On-Ramp		1							
Acceleration lane length	L <sub>A</sub>	100	m						
Distance to adjacent downstream Ramp	L <sub>dow n</sub>	N/A	m	applicable for	Equation 3	only			
Distance to adjacent upstream Ramp	L <sub>up</sub>	N/A	m	applicable for	Equation 2	only			
Passenger car equivalent for truck (HV factor)	Et	2	Given by SWTC	C. Cross check w	know n.				
Proportion of approaching freeway flow									
remaining in Lanes 1 and 2 immediately upstream of merge	<b>_</b>		6 Lane FW	0.5007					
Intermediate speed determination variable for	P <sub>FM</sub>	0.587	O Lane FVV	0.5867	Equation 1				
merge area	Ms	0.326							
Space Mean Speed in Ramp Influence	INIS	0.020							
Area (450m)	S <sub>R</sub>	96.0	km/h	87%	of FFS				
Number of out side lane	No	1.0							
Ave. per-lane flow rate in outer lanes	V <sub>OA</sub>	1845.5	<500pc/h	>500<2300pc/h	>2300pc/h				
Space Mean Speed in outer lanes	S <sub>0</sub>	102.2			104.0251				
	-0								
Conversion to pc/h under base conditions							No.of Lane		
pc/h	v	PHF	%HV	f <sub>H∨</sub>	fp	v			
VF	4039	0.95	5.03	0.952	1	4465			
V <sub>R</sub>	244	0.95	0.00	1.000	1	257			
V <sub>12</sub>						2620			
V <sub>R12</sub>						2877			
V <sub>FO</sub>						4722			
Merge									
Density of merge influence area	D <sub>R</sub>	15.87	pc/km/ln						
Level of Service		С							
		V <sub>F</sub>	V <sub>R</sub>	V <sub>FO</sub>					
Capacity Check									
Density as per Graph (Exhibit 11.6) of HCM									
2010 - pc/ln/ speed	Density	13.53							
Level of Service V/C		0.63							
FW/Ramp max. lane capacity based on speed		2350							

# I. Inventory of Existing Signs

### **Existing Signs Inventory – Mooney Mooney**



M1\_NB\_01



M1\_NB\_05



M1\_SB\_02



M1\_SB\_06



M1\_NB\_02



M1\_NB\_06



M1\_SB\_03



MMPI\_01



M1\_NB\_03



M1\_NB\_07



M1\_SB\_04



MMPI\_02



M1\_NB\_04



M1\_SB\_01



M1\_SB\_05



MMPI\_03



MMPI\_04



MMPI\_08



MMPI\_12



MMPI\_16



MMPI\_05



MMPI\_09



MMPI\_13

Moaney 👪 Brooklyn

MMPI\_17



MMPI\_06



MMPI\_10



MMPI\_14





MMPI\_07



MMPI\_11



MMPI\_15



MMPI\_19



MMPI\_18









MMPI\_22



MMPI\_23

### J. NOT USED

# **K.** Authority Correspondence



Mr Brian Glendenning Chief Executive Officer Central Coast Council PO Box 21 GOSFORD NSW 2250

#### Attention: Robert Drew

#### Planning Proposal – Peat Island / Mooney Mooney

Dear Mr Glendenning,

Thank you for your correspondence dated 21 September 2017 requesting Transport for NSW (TfNSW) review and comment on the Planning Proposal for the subject site. Please accept this as a joint response with Roads and Maritime Services, collectively referred to as TfNSW.

TfNSW's primary interests are in the road network, traffic and broader transport issues. In particular, the efficiency and safety of the classified road network, the security of property assets and the integration of land use and transport.

In accordance with the *Roads Act 1993*, Roads and Maritime has powers in relation to road works, traffic control facilities, connections to roads and other works on the classified road network. The Pacific Motorway (M1) is a classified (State) road and is a critical freight and transport route within NSW. Roads and Maritime is the Roads Authority for the M1. Central Coast Council is the Roads Authority for all other roads from the Interchange.

In addition to the above, Roads and Maritime is the proprietor of freehold land within the area nominated in the Planning Proposal and also has operational interests in land adjoining the M1.

The subject planning proposal seeks to rezone surplus government land at Mooney Mooney from SP2 Hospital, SP2 Educational Establishment and RE1 Public Recreation to R1 General Residential, R2 Low Density Residential, B2 Local Centre, RE1 Public Recreation and E1 National Park and Nature Reserves. The proposal allows for residential development, community facilities, a neighbourhood retail centre, recreation, a marina, relocation of existing emergency service facilities and addition of land to Popran National Park.

The available information has been reviewed and the following comments are provided for consideration.

#### Traffic and Transport Assessment

The Traffic and Transport Review prepared by Mott MacDonald in September 2016 should be updated to provide an assessment of the Mooney Mooney Interchange and impacts of the proposed rezoning. The update should be completed in accordance with the *Guide to Traffic Generating Developments* (NSW RTA 2002) and supplements and should include but not be limited to, the following:

• An assessment of the traffic and safety implications at the Mooney Mooney Interchange, including a review of impacts commercial development to the west of the M1 may have on Interchange operations. It is recommended that a detailed traffic assessment be undertaken, which includes (but not limited to) a microsimulation analysis of the operation of the on and off ramps. Further consultation with Roads and Maritime is recommended to seek advice regarding more detailed scope of work and model specification.

- The 5 studies noted for completion at Section 6 *Key Findings and Recommendations* of the *Traffic and Transport Review* should be included in the updated assessment.
- Consideration of the traffic impacts on the existing intersections and the capacity of the road network to safely and efficiently cater for the additional vehicular traffic generated.
- Identify the necessary road network infrastructure upgrades the area requires to maintain existing levels of service on both the local and classified road network. Any proposed changes to the road network will need to be discussed with Council and Roads & Maritime and be supported by a Road Safety Evaluation.
- Any other impacts upon the road network including consideration of pedestrian, cyclist and public transport facilities including commuter car parking at local railway stations.
- Identify feasible options to modify transport impacts if required.

#### Environmental Assessment

As a green field site offers the greatest flexibility to manage impacts associated with sensitive land uses, it is important that the compatibility of the proposed zones in the vicinity of the M1 Pacific Motorway be considered. The following assessments should be provided for RMS consideration:

- An updated noise impact assessment validated by noise measurements at locations where noise levels are highest and where land is nominated for the most sensitive land uses (i.e. zones where it is expected to be developed for habitable occupation). An assessment should include details of necessary treatments, as required, based on the noise levels to demonstrate that the future land use can implement basic mitigation measures to manage noise in accordance with relevant regulations and standards. Where relevant, the report should nominate any infrastructure upgrades to ensure the proposed rezoned land for future habitable development will not be adversely affected by noise.
- An illumination report identifying whether the Planning Proposal will be adversely affected by light spill created by the lighting of the M1 Pacific Motorway, and nomination of any mitigation measures considered necessary to ameliorate identified adverse impacts.

#### Property Advice

The property has a common boundary with the Pacific Motorway which has been declared as Freeway. Direct access for any lot across this boundary is restricted. Public access points to the Pacific Highway and Peats Ferry Road are available as shown on the attached Motorway Plan.

TfNSW notes that the strategic concept plan incorporates the relocation of existing RMS operations. It is recommended that NSW Property further consult directly with RMS to satisfy other matters relating to impacts affecting existing RMS operations. Please contact Kylie-Anne Pont, Development Assessment Officer, via email <u>development.hunter@rms.nsw.gov.au</u> to arrange a meeting with relevant stakeholders.

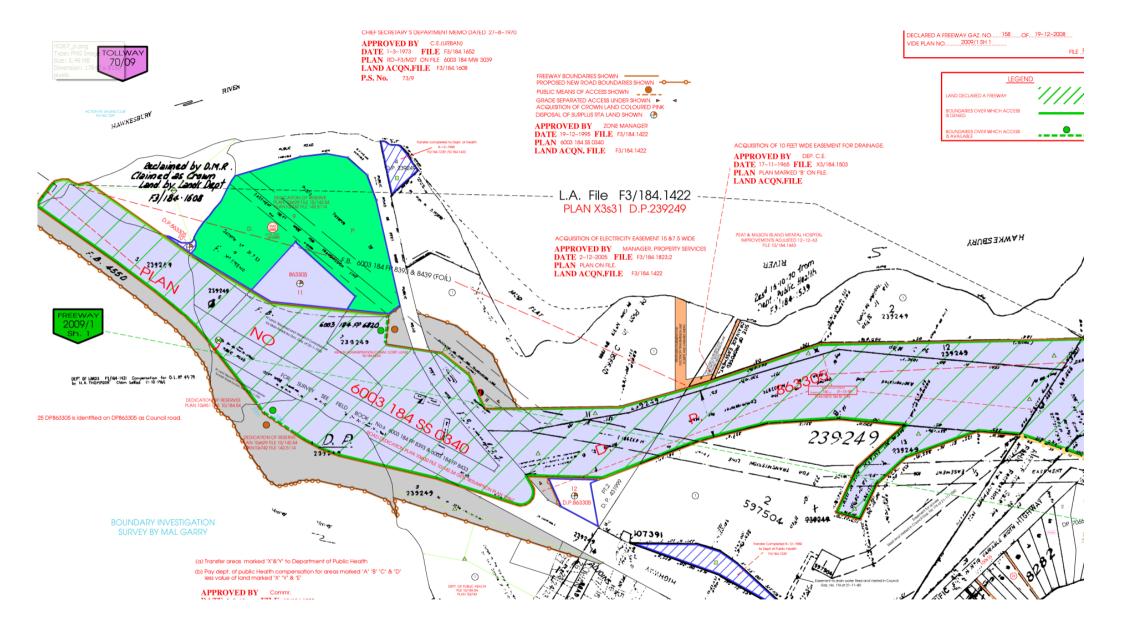
Thank you again for requesting TfNSW comment on this proposal. If you have any further questions, Mr Lee Farrell, Transport Planner at TfNSW, would be pleased to take your call on (02) 8265 9943. I hope this has been of assistance.

Yours sincerely

14/5/2018

Mark Özinga Principal Manager, Land Use Planning and Development Freight, Strategy and Planning Division

> CD17/11134 CR2016/005524



Infrastructure Property Comments - Mooney Mooney - 18 August 2014.doc

### Loder, Thomas

From:PONT Kylie-Anne <Kylie-Anne.PONT@rms.nsw.gov.au>Sent:Thursday, September 27, 2018 4:15 PMTo:Lee, Greg ACc:Farrell, LeeSubject:FW: Mooney Mooney - Traffic Study

#### Hi Greg,

Roads and Maritime's Road Network Analysis team have reviewed your email below and provided the following comments:

Methodology:

Given the proposed development scope, and the existing traffic, the SIDRA modelling is recommended for the local road intersection traffic performance assessment.

The Highway Capacity Manual ('HCM') desktop analysis is recommended for the M1 interchange ramp terminals (exits and entries) assessment.

Alternatively, VISSIM microsimulation modelling can be used for the M1 interchange analysis providing that the reported ramp performance measures will be the same or similar to HCM measures. VISSIM models can also be used to extract intersections' traffic performance measures.

<u>Scope - Peak hours relevant for the assessment:</u>

For M1 Motorway – select the AM peak hour when M1 southbound traffic is at the highest (peak) flow, PM peak hour when M1 northbound traffic is at the peak flow. In a similar way, determine weekend 'midday' peaks for M1 both (southbound and northbound) directions. For the nominated three intersections: select AM, PM and weekend peak hours when the intersections' total flow is the highest. Note, traffic counts may reveal other peak traffic hours relevant to the analysis and assessment. Furthermore, the assessment should also have consideration for future traffic generation for the ultimate development potential, including an outline of background growth and the cumulative traffic impacts for 25 years over 5 year increments.

Further to the above comment, see the additional 'red' comments below in direct response to each of the points raised in your email.

#### Regards,

Kylie-Anne Pont Development Assessment Officer Network & Safety Hunter | Regional & Freight T 02 4908 7683 M 0475 989 994 www.rms.nsw.gov.au Every journey matters

Roads and Maritime Services Level 8, 266 King St Newcastle NSW 2300

From: Lee, Greg A [mailto:Greg.Lee@mottmac.com] Sent: Thursday, 20 September 2018 3:18 PM To: Development hunter Cc: Farrell, Lee; Stephens, Matthew; Zaaiter, Ghada; Loder, Thomas Subject: RE: Mooney Mooney - Traffic Study

Hi Kylie-Anne,

Thank you for your comments below regarding the Mooney Mooney development, apologies it has been a while since our last email exchange, we have since been working with our client to bed down scope and approach. We have agreed with the Client to undertake VISSIM modelling of the study area as opposed to the SIDRA analysis. If possible before we jump into the modelling and obtain traffic counts we would like to discuss and where possible agree in principle to our broad methodology.

Below are the major assumptions considered for the development of the VISSIM model for the Mooney Mooney Planning Proposal.

- 1. The RMS permanent count data from F3FWY003 and 76001 will be used to establish the traffic flow along the M1 and Pacific Highway, distinguishing between heavy and light vehicles. This data will also serve as a guide to determine timeframes for data collection. OK.
- 2. Classified video intersection counts will be conducted at three intersections within the site as per the diagram below. As stated above, the RMS permanent counters have been used to determine the timeframes for collection (refer to diagrams attached).
  - 1. 5:00am 8:00am for AM weekday
  - 2. 3:00pm 6:00pm for PM weekday
  - 3. 8:00am 6:00pm for weekend peak
  - 4. Count M1 Interchange ramp existing and entering traffic in the same time periods as the intersections' counts



- 3. Three 7x7 'base case' demand matrices (2-hrs = 1-hr lead-in and 1-hr peak) and three 'development case' demand matricies will be developed using both the RMS permanent count data and classified survey as per the diagram below. Given the modelled network extent, the time duration of 1.5 hour (30min 'warm-up' and one peak hour) is enough. And preferable, for each model set 15min matrices (4 matrices) for each peak hour. RMS has no preference whether 15min or 30min matrices are used for the "warm up" and "cool down" periods.
- 4. Demand matrices will be developed using the IPF (Frataring) matrix scaling method, against observed cordon flows. I am not sure how matrix (Fratar) scaling is relevant here. Estimate traffic matrices for the existing traffic scenario from the traffic counts. Make sure that the existing models replicate the M1 traffic flows and the intersections' turning movements.

Future traffic matrices should comprise the development generated traffic plus 'background traffic'. The 'background' traffic should be for the year when the developments are likely to be completed. Determine the 'background' traffic growth from RMS's STFM model, for AM and PM peak periods. Since the STFM does not have future weekend traffic, assume a weekend traffic growth rate similar to the weekday traffic growth rate – other estimation methods by the Consultants may also be considered but should be justified. Distribute ('in' and 'out') the development generated traffic to zones 1, 5, 6 and 7, proportionally to their demand (production/attraction) volumes.



- 5. Six VISSIM models, three 'base case' and three 'development case', will be developed using the information above and coded based on a spatially accurate aerial photo. Minimum 8 models (2 x AM, 2 x PM (weekdays), 4 x weekend (Sat and Sun)) over 25 years in 5 year increments.
- 6. Zones 1, 5 and 7 will extend up to 500m beyond the interchange ramps in each direction. Extend the modelled M1 motorway (zones 1 & 5) beyond the interchange ramps for at least 1km.
- 7. Given the small scale, mostly free-flow nature of the model travel time calibration is not proposed. Agreed.
- 8. This will be a 2D model. Acceptable.
- 9. Model the motorway's existing S/B and N/B parking,

Please advise if this scope and methodology is acceptable. We are happy to discuss any of the above.

With Kind regards,

Greg Lee Principal Civil Engineer



Mott MacDonald 383 Kent Street Sydney NSW 2000 PO Box Q1678, QVB Sydney, NSW 1230 Australia

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From: Development hunter [mailto:Development.hunter@rms.nsw.gov.au] Sent: Thursday, 23 August, 2018 12:42 PM To: Lee, Greg A <Greg.Lee@mottmac.com> Cc: Farrell, Lee <Lee.Farrell@transport.nsw.gov.au> Subject: FW: Mooney Mooney - Traffic Study

#### Hi Greg,

I refer to your email below seeking clarification about the electronic analysis to be submitted with the Planning Proposal at Mooney Mooney.

The M1 Pacific Motorway is a critical piece of State road infrastructure facilitating the efficient flow of high volumes of freight and passenger vehicle traffic between Gosford and Newcastle to/from Sydney and beyond. Furthermore the network provides dedicated rest stops for both heavy vehicles and light vehicles promoting safe use of the high speed road network.

The complexity and sensitivity of vehicle movements at the Mooney Mooney interchange is heavily influenced by the high traffic volumes, high speed environment, close proximity of the low speed internal local road network intersections and exiting rest stop facilities on both east and west sides of the M1 Pacific Motorway. As such, it is critical that a comprehensive assessment of the build-up and dissipation of queues and their effect on surrounding congestion and travel times is sensitively modelled to ensure the impacts of the planning proposal on the M1 Pacific Motorway are identified and suitably addressed within a Traffic Impact Assessment. Neither Transport for NSW nor Roads and Maritime support the use of a micro-analytical tool like SIDRA for the analysis of this proposal.

Kylie-Anne Pont Development Assessment Officer Network & Safety Hunter | Regional & Freight T 02 4908 7683 M 0475 989 994 www.rms.nsw.gov.au Every journey matters

Roads and Maritime Services Level 8, 266 King St Newcastle NSW 2300

From: Lee, Greg A [mailto:Greg.Lee@mottmac.com] Sent: Tuesday, 21 August 2018 5:56 PM To: MARLER Peter C Cc: Gregg, Amy; Loder, Thomas; Higgisson, Rachel Subject: Mooney Mooney - Traffic Study

#### Hi Peter,

Greg Sullivan from the Department of Planning mentioned it may be worthwhile touching base with yourself regarding a Planning Proposal we are working on with Property NSW at Mooney Mooney. We are currently undertaking a Traffic and Transport Assessment of PNSW's Mooney Mooney Site, which they are ultimately looking to divest. I am not sure if you are familiar with the site at all? We are hoping to seek some clarification around scope of works.

We've received the attached joint response from TfNSW/RMS. If possible we would like to discuss the scope and detail, in particular the below request for a microsim model of the on and off ramps.

"An assessment of the traffic and safety implications at the Mooney Mooney Interchange, including a review of impacts commercial development to the west of the M1 may have on Interchange operations. It is recommended that a detailed traffic assessment be undertaken, which includes (but not limited to) a microsimulation analysis of the operation of the on and off ramps. Further consultation with Roads and Maritime is recommended to seek advice regarding more detailed scope of work and model specification."

The proposed development includes the creation of 250 units and approx. 90 bedroom hotel. Given the nature of the area and the expected traffic generation we are proposing to undertake an assessment of the site using SIDRA as per our attached scope. This will still allow us to assess the ramps and queueing from the first intersections from the off ramps.

If possible it would be good to have a quick chat regarding the above as I understand you may not be familiar with the project and there may be a better point of contact at RMS.

With Kind Regards,

Greg Lee Principal Civil Engineer



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